

OHIO TURNPIKE COMMISSION

ADDENDUM NO. 1

CONTRACT NO. 77-13-01

**THIRD LANE CONSTRUCTION
M.P. 59.52 TO M.P. 64.13
LUCAS AND WOOD COUNTIES, OHIO**

OPENING DATE: 2:00 P.M. (E.S.T.), DECEMBER 20, 2012

ATTENTION OF BIDDERS IS DIRECTED TO:

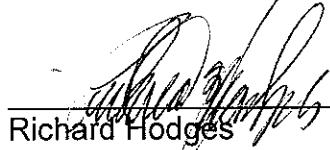
**ANSWERS TO QUESTIONS RECEIVED THROUGH
12:00 P.M., DECEMBER 10, 2012**

MODIFICATIONS TO THE BID FORM
Pages revised: OTC-BF-2

MODIFICATIONS TO THE SPECIFICATIONS
Pages revised: TOC-2, SP-283 and SP-284
Pages added: SP-256A, SP-256B, SP-256C, SP-256D, SP-256E and SP-256F

MODIFICATIONS TO THE DRAWINGS
Contract Drawings: Sheets 1, 12, 17, 42 and 118 of 322
OTC Standard Drawings: AS-1, AS-2, AS-3, AS-4, AS-5 and CB-1
ODOT Standard Drawings: BP-2.1, BP-2.2 and BS-1-93

Issued by the Ohio Turnpike Commission on December 10, 2012. Issuance authorized by Richard Hodges, Executive Director and Kathleen Weiss, General Counsel.


Richard Hodges 12/10/12
Date


Kathleen Weiss 12/10/12
Date

**OHIO TURNPIKE COMMISSION
ADDENDUM NO. 1
CONTRACT NO. 77-13-01**

ANSWERS TO QUESTIONS RECEIVED THROUGH 12:00 P.M., DECEMBER 10, 2012

- Q#1** Plan sheet 8 shows shoulder details requiring a saw cut per 203.04e where new asphalt meets existing but there is no pay item. Should there be a separate pay item for this, or is this considered an “incidental” cost?
- A#1** *Saw cutting is an incidental cost in accordance with 203.04(E) of the ODOT CMS, and no separate pay items shall be included.*
- Q#2** Due to the complexity of the bid items for the above named project bidding on the 20th, I was hoping you would consider forwarding the excel file for the bid items so that I may import it into our bidding software rather than have to manually enter this. Please advise.
- A#2** Yes, the Bid Form in an electronic spreadsheet will be e-mailed in response to requests received by Commission's Procurement Manager at kevin.golick@ohioturnpike.org.
- Q#3** SP 115 Railroad Protective Liability Insurance and details provided in SP 827B (Norfolk-Southern) and SP 827D (CSX) does not include all information for the insurance carriers to quote the premium for the insurance required. Please provide the following additional information for both the Norfolk-Southern and CSX Railroads: Number of passenger trains per day. Max authorized operating speed of passenger trains (MPH). Number of freight trains per day. Max authorized operating speed of freight trains (MPH).
- A#3** *The Commission is endeavoring to obtain this information and will provide the answer with Addendum No. 2.*
- Q#4** SP 622 Portable Concrete Barrier. Are portable concrete barriers owned by the Commission currently stored at various interchanges available for use by the Contractor? (yes or no). If yes, please specify locations and quantities available.
- A#4** *No. Commission owned portable concrete barriers are not available for the Contractor's use in this Project.*

Q#5 Is ZoneGuard portable steel barrier an approved barrier type? (yes or no)

A#5 *A portable steel barrier system specification has been added via this Addendum No. 1 to the "Material" section of the "SP 622 – Concrete Portable Barrier" Special Provision.*

Q#6 SP 104 Access to Turnpike and Restrictions states no work shall be permitted on the railroad bridges at MP 61.5 (NS RR) and MP 63.5 (CSX RR) until agreements by the Commission are finalized with an anticipated agreement date of November 2, 2013. At MP 61.5 (plan sheet 263) may bridge work proceed prior to the agreement in areas not adjacent to the NS RR? (Specifically, no work on piers 1 & 2 until an agreement on or about November 2, 2013 BUT proceeds with abutments and piers 3 & 4 outside the NS RR limits in Phase 2 construction (2013).

A#6 *Any Work within, or that requires access to, a railroad right-of-way shall not begin until the necessary railroad agreements are finalized with the Commission and all required submittals for both railroads and the Commission are approved.*

Q#7 Similarly, at MP 63.5 (plan sheet 290) may work proceed in Phase 2 construction (2013) on abutments outside the CSX limits prior to the agreement?

A#7 *See the response to Q#6.*

Q#8 Regarding the above referenced project, in order to get a quote for the Railroad Protective Insurance, I need the following information for each railroad. Number of Trains per day, Passenger ?, Freight ?, Unscheduled ? and Scheduled ? I need this information for both Norfolk Southern and CSX Railroad.

A#8 *See the response to Q#3.*

Q#9 The notes on page 12 regarding the Concrete Barrier, Type B-50 & Type C-50 give two different methods for curing the barrier wall. The Type B-50 calls for a clear cure/sealing compound while the Type C-50 calls for curing per 511.17, Method B (membrane curing). In addition OTC Standard Drawing CBR-3 calls for concrete surfaces to receive a masonry coat paid in accordance with SP 536A (see note 10). Please clarify what curing method is required and whether the masonry coat will be used. Currently the

only quantity of masonry coat setup in the bid is limited to the Median Wall, 2 ft & 4 ft (see bid item #31).

- A#9 *All concrete barrier surfaces shall be cured using the SP 536A, Masonry Coating in accordance with the manufacturer's recommendations. With this Addendum No. 1, the SP 536A – Masonry Coating Specification has been added to the Special Provisions, a note has been added to the Plan's General Notes, and a quantity has been added to the Plan's General Summary and to the Bid Form.*
- Q#10 **The EB & WB bridges over the Maumee River have already been widened. Currently, portable concrete barrier is in place on each bridge closing off the inside third lane. Can this portable concrete barrier remain and be used by the contractor in the MOT phasing? (yes or no) If yes, what is to be done with this portable concrete barrier upon completion of the project? If no, what is to be done with this portable concrete barrier during the project?**
- A#10 *No, the existing portable barrier over the Maumee River Bridge may not be used during this Project. The Commission will remove the concrete barrier from the project site at the Contractor's request. The Contractor will need to then place its portable concrete barrier as needed, once the existing concrete barrier is removed. The "for information purposes only" quantity of Item 622 - Portable Concrete Barrier on Sheet 17 of 322 has been revised to 10.72 miles via this Addendum No. 1.*
- Q#11 **Maintenance of Traffic plan sheets 32, 32A, 33 and 34 show details for an optional winter phase. These sheets show Item 615 Temporary Pavement, Class B and Item 615 Temporary Roads. There is no pay item in the proposal for this work. Please add these items of work or explain how this is to be paid for.**
- A#11 *Should the Contractor decide to use the optional winter phase in order to gain access to a bridge or bridges during the winter of 2013-2014, then the additional Maintenance of Traffic Work will be required, including all temporary pavement and access, which shall be incidental to item 614 – Maintaining Traffic.*
- Q#12 **Will the contractor be required to follow ODOT Spec 451.12 and 451.13 from the ODOT CMS book which requires smoothness testing and grinding for corrections on the 10" Reinforced Concrete Pavement Item?**
- A#12 Yes.

Q#13 The special provisions (SP 115) do not include railroad train traffic information for either NSRR or CSX. Please provide the following information in order that we can obtain quotes for the Railroad Protective Insurance Coverages (bid items 584 and 585): 1. Are passenger trains on the track? 2. How many trains / day? 3. What is the average speed of the trains?

A#13 See the response to Q#3.

Q#14 Will 702.13 rubberized asphalt emulsion (tack coat) be required over the new concrete base pavement?

A#14 Yes, tack coat conforming to SP 407 shall be required over concrete base pavement per the Plans and the Specifications.

Q#15 During Phase 1 construction, will the contractor be allowed to operate equipment / trucks in the driving lane (without a buffer lane)?

A#15 During the shoulder reconditioning process, the Contractor shall move traffic into the existing passing lane while utilizing a driving lane closure. This closure shall be implemented per OTC Standards Drawings, and the shoulder reconditioning Work will be performed using this closure. The lane closure shall be removed and reset each day, and the elevation difference between the driving lane and the shoulder Work shall not exceed three (3) inches. The closed driving (right) lane shall serve as a buffer between the traveling lane and the Work locations. The Contractor may use the driving lane to transport men, equipment, and materials as necessary.

Q#16 During Phase 1 construction, will grinding and paving trucks be allowed to ingress/egress into the closed zone at the point of milling / paving operations OR will the ingress be at the beginning of the zone closure and egress at the end of the zone closures?

A#16 Ingress and egress locations can be located where requested by the Contractor, but the approval is always at the discretion of the Chief Engineer. These ingress and egress locations shall be in accordance with OTC Standard Drawings, which require the use of flaggers at each location.

Q#17 According to Note #8 on CBR-3, 2.5 ft will be deducted from the barrier wall quantity and paid under with each junction box. MCC-1 & MCC-2 then

show 12 ft of barrier wall between expansion joints for 2 junction boxes (staggered at 8 ft) or 4 ft of barrier wall for 1 junction box. Please clarify how much barrier wall will be deducted at each junction box.

A#17 *The Commission will provide the answer with Addendum No. 2.*

Q#18 Bid Item 77- Class C Concrete, Miscellaneous: there is no plan detail, what is the purpose for the multiple locations of this item in the drainage quantities?

A#18 *The Commission will provide the answer with Addendum No. 2.*

Q#19 Can you please post the excel spreadsheet for the bid items on this project? Thanks.

A#19 *See answer to Q#2.*

Q#20 I see this is designated with "13", does that not make it a 2013 project? Can you delay this project until 2013?

A#20 *The Commission will provide the answer with Addendum No. 2.*

Q#21 Would OTC please provide a bid excel spreadsheet in lieu of requiring the contractor to ink in all the bid items?

A#21 *See response to Q#2. A new bid form is provided with this Addendum No. 1.*

Q#22 Will the OTC allow batch plants to be setup on any of their property within or outside of the work limits for this project (interchange areas for example)?

A#22 *The Commission will provide the answer with Addendum No. 2.*

MODIFICATIONS VIA ADDENDUM NO. 1 TO THE CONTRACT DOCUMENTS FOR CONTRACT NO. 77-13-01

The following changes are made to the Contract Documents for Contract No. 77-13-01:

(BIDDERS ARE ADVISED TO UTILIZE THE ATTACHED REPLACEMENT PAGES AND PLAN SHEETS).

MODIFICATIONS TO THE BID FORM

Deletions are shown with strikethrough text.

Changes/Additions are shown with ***bold italicized*** text.

Page OTC-BF-2

The Item No. SP-536 associated with Bid Ref. No. 34, CONCRETE WEATHERPROOFING, MEDIAN WALL, approx. quantity 2990, on the Bid Form has been deleted and replaced with ***Ref. No. 31A SP536A MASONRY COATING***, with an approximate quantity of ***31,489 SQ.YDS.***

MODIFICATIONS TO THE SPECIAL PROVISIONS

Deletions are shown with strikethrough text.

Changes/Additions are shown with ***bold italicized*** text.

Page Table of Contents 2 of 4:

Add new Specification: ***SP 536A – MASONRY COATING, Pages 256A-256F.***

Pages SP-256A through SP-256F:

A new Special Provision "***SP 536A – MASONRY COATING***" was added to the Contract Documents as new Pages ***256A, 256B, 256C, 256D, 256E and 256F***.

Pages SP-283 and SP-284

SP 622 PORTABLE CONCRETE BARRIER: Section B entitled "Material" is revised as follows:

B. Materials

Portable concrete barrier shall be as specified in Section 622.02 of the Specifications.

Portable Concrete Barrier Option 1: Portable concrete barrier in accordance with this section, ODOT CMS 622 Portable Concrete Barrier and ODOT Standard Drawings RM-4.3 32" Portable Concrete Barrier.

Portable Concrete Barrier Option 2: A portable steel barrier system comprised of nominally eight (8) gauge thick ASTM A36 pressed steel panels, galvanized prior to assembly. Barrier sections shall be 50 feet long and 32 inches in overall height. Base width shall be a minimum of 2.3 feet and a top width of 6.25 inches. The barrier shall have a mountable foot section and have rubber feet fastened to the sections at 4 foot intervals. A lifting system shall be incorporated into the sections. Installation of the sections shall not impede drainage. Each section shall be anchored to the pavement at a minimum of two points. Portable steel barrier shall meet the requirements of NCHRP Report 350 Test Level 3 (TL3) and Test Level 4 (TL4).

MODIFICATIONS TO THE PLAN DRAWINGS

Modifications to the Plan Drawings

Deletions in Plan Notes are shown with strikethrough text.

Changes/Additions in the Plan Notes are shown with ***bold italicized*** text.

Additions and deletions on Plan Drawings are indicated with a cloud and revision triangle thus:



Plan Sheet 1 of 322

The OHIO TURNPIKE COMMISSION STANDARD DRAWINGS table was revised as follows:

AS-1	01-24-11 <i>11-20-12</i>
AS-2	01-24-11 <i>11-20-12</i>
AS-3	01-24-11 <i>11-20-12</i>
AS-4	01-24-11 <i>11-20-12</i>
AS-5	01-24-11 <i>11-20-12</i>
RPM-1	06-25-07 <i>12-21-11</i>
TCR-2	06-25-07 <i>12-21-11</i>

The OHIO DEPARTMENT OF TRANSPORTATION STANDARD DRAWINGS table was revised as follows:

TC-22.10 ~~01-19-07~~ ***01-19-01***

Plan Sheet 12 of 322:

The following General Note was added:

SP 536A, MASONRY COATING

ALL EXPOSED CONCRETE BARRIER SURFACES SHALL RECEIVE A COATING PER SP 536A. THE MASONRY COATING SHALL ALSO BE USED AS THE CURE COAT. THE FINAL APPEARANCE SHALL BE UNIFORM AND CONSISTENT, AND SHALL CONFORM TO ALL PROVISIONS IN SP 536A. NO ADDITIONAL PAYMENT SHALL BE MADE IF THE CONTRACTOR IS REQUIRED TO APPLY ADDITIONAL COATINGS TO ACHIEVE A UNIFORM APPEARANCE. THE MATERIAL SHALL BE APPLIED PER THE MANUFACTURER'S RECOMMENDATIONS. ALL LABOR, EQUIPMENT, AND MATERIALS NECESSARY TO COMPLETE THIS WORK SHALL BE INCLUDED IN THE BID PRICE FOR SP 536A, MASONRY COATING. THE FOLLOWING QUANTITY IS INCLUDED IN THE GENERAL SUMMARY FOR SP536A, MASONRY COATING:

SP 536A, MASONRY COATING

31,489 SQ.YD.

The following General Notes were revised to state as follows:

ITEM 622 - CONCRETE BARRIER, TYPE B-50, AS PER PLAN

ITEM 622 - CONCRETE BARRIER, TYPE B-50, AS PER PLAN SHALL BE CONSTRUCTED IN ACCORDANCE WITH OTC STANDARD DRAWING CBR-3 AND CMS 622.

IN LIEU OF THE CURING COMPOUNDS SPECIFIED IN SECTION 622.07 OF THE SPECIFICATIONS, THE CONCRETE BARRIER SHALL BE CURED USING THE MATERIAL SPECIFIED IN SP 536A. THE SP 536A MATERIAL APPLICATION SHALL BE AS PER THE RECOMMENDATIONS OF THE MANUFACTURER. THE CONTRACTOR SHALL SUBMIT TECHNICAL DATA FOR THE SP 536A MATERIAL TO THE ENGINEER FOR APPROVAL. THE COST OF CURING THE WALL AND BARRIER SHALL BE INCLUDED IN THE BID PRICE FOR SP 536A, MASONRY COATING. ALL OTHER PROVISIONS OF SECTION 622 OF THE SPECIFICATIONS SHALL APPLY.

ITEM 622 - CONCRETE BARRIER, TYPE C-50, AS PER PLAN

ITEM 622 - CONCRETE BARRIER, TYPE C-50, AS PER PLAN SHALL BE CONSTRUCTED IN ACCORDANCE WITH OTC STANDARD DRAWING CBR-3, AND SECTION 622 OF THE SPECIFICATIONS.

IN LIEU OF THE CURING COMPOUNDS SPECIFIED IN SECTION 622.07 OF THE SPECIFICATIONS, THE CONCRETE BARRIER SHALL BE CURED USING THE MATERIAL SPECIFIED IN SP 536A. THE SP 536A MATERIAL APPLICATION SHALL BE AS PER THE RECOMMENDATIONS OF THE MANUFACTURER. THE CONTRACTOR SHALL SUBMIT TECHNICAL DATA FOR THE SP 536A MATERIAL TO THE ENGINEER FOR APPROVAL. THE COST OF CURING THE WALL AND BARRIER SHALL BE INCLUDED IN THE BID PRICE FOR SP 536A, MASONRY COATING. ALL OTHER PROVISIONS OF SECTION 622 OF THE SPECIFICATIONS SHALL APPLY.

ITEM 622 - CONCRETE BARRIER, TYPE D, AS PER PLAN

ITEM 622 - CONCRETE BARRIER, TYPE D, AS PER PLAN SHALL BE CONSTRUCTED IN ACCORDANCE WITH OTC STANDARD DRAWING CBR-3, AND SECTION 622 OF THE SPECIFICATIONS.

IN LIEU OF THE CURING COMPOUNDS SPECIFIED IN SECTION 622.07 OF THE SPECIFICATIONS, THE CONCRETE BARRIER SHALL BE CURED USING THE MATERIAL SPECIFIED IN SP 536A. THE SP 536A MATERIAL APPLICATION SHALL BE AS PER THE RECOMMENDATIONS OF THE MANUFACTURER. THE CONTRACTOR SHALL SUBMIT TECHNICAL DATA FOR THE SP 536A MATERIAL TO THE ENGINEER FOR APPROVAL. THE COST OF CURING THE WALL AND BARRIER SHALL BE INCLUDED IN THE BID PRICE FOR SP 536A, MASONRY COATING. ALL OTHER PROVISIONS OF SECTION 622 OF THE SPECIFICATIONS SHALL APPLY.

THE COST OF THE BARRIER TRANSITION SHALL BE INCIDENTAL TO ITEM 622 - CONCRETE BARRIER, TYPE D, AS PER PLAN. THE BARRIER TRANSITION SHALL BE CONSTRUCTED IN ACCORDANCE WITH DETAIL SHEET 195B. PAYMENT FOR ALL MATERIALS AND LABOR SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 622 - CONCRETE BARRIER, TYPE D, AS PER PLAN.

Plan Sheet 17 of 322:

The Quantity for Item 622, Concrete Barrier Delineator, Phase 2, MP 59.28 to MP 64 36 EB have been revised from 2493 to **2682** Each.

The Quantity for Item 622, Temporary Concrete Barrier, Phase 2, MP 59.28 to MP 64.36 EB have been revised from 4.72 to **5.08** Miles.

The Quantity for Item 622, Concrete Barrier Delineator, Phase 2, MP 59.24 to MP 64.88 WB have been revised from 2461 to **2978** Each.

The Quantity for Item 622, Temporary Concrete Barrier, Phase 2, MP 59.24 to MP 64.88 WB have been revised to from 4.66 to **5.64** Miles

The Total Quantity for Item SP 622, Concrete Barrier Delineator, is modified from 4,954 to **5,660** Each.

The Total Quantity for Item SP 622, Portable Concrete Barrier, is modified from 9.38 to **10.72** Miles.

Plan Sheet 42 of 322

The following General Summary item has been revised:

~~SP 536, WEATHERPROOFING MEDIAN WALL, 2990 SQ.YD.~~ was deleted and replaced with **SP 536A, MASONRY COATING, 31,489 SQ.YD.**

Plan Sheet 118 of 322

~~SP 536, WEATHERPROOFING MEDIAN WALL~~ has been deleted from the Sub-Summary for Median Walls table.

SUBSTITUTIONS OF OTC STANDARD DRAWINGS

The OTC Standard Drawing AS-1 shall be replaced with new OTC Standard Drawing AS-1 (dated NOVEMBER 20, 2012).

The OTC Standard Drawing AS-2 shall be replaced with new OTC Standard Drawing AS-2 (dated NOVEMBER 20, 2012).

The OTC Standard Drawing AS-3 shall be replaced with new OTC Standard Drawing AS-3 (dated NOVEMBER 20, 2012).

The OTC Standard Drawing AS-4 shall be replaced with new OTC Standard Drawing AS-4 (dated NOVEMBER 20, 2012)

The OTC Standard Drawing AS-5 shall be replaced with new OTC Standard Drawing AS-5 (dated NOVEMBER 20, 2012).

The OTC Standard Drawing CB-1 shall be replaced with new OTC Standard Drawing CB-1 (marked REVISED FOR ADDEDUM NO. 1 DATED: 12-10-2012).

SUBSTITUTIONS OF ODOT STANDARD DRAWINGS

The ODOT Standard Drawing BP-2.1 shall be replaced with new ODOT Standard Drawing BP-2.1 (dated 7-18-08).

The ODOT Standard Drawing BP-2.2 shall be replaced with new ODOT Standard Drawing BP-2.2 (dated 7-18-08).

The ODOT Standard Drawing BS-1-93 shall be replaced with new ODOT Standard Drawing BS-1-93 (dated 12-19-94).

ATTACHMENTS:

Bid Form Page OTC-BF-2

Special Provisions Table of Contents Page 2 of 4

Special Provisions Pages SP-256A, SP-256B, SP-256C, SP-256D, SP-256E, SP-256F, SP-283 and SP-284

Plan Sheets Pages 1, 12, 17, 42 and 118 of 322

OTC Standard Drawings AS-1, AS-2, AS-3, AS-4, AS-5 and CB-1

ODOT Standard Drawings BP-2.1, BP-2.2 and BS-1-93

Receipt of Addendum No. 1 to Contract
No. 77-13-01 is hereby acknowledged:

(Firm Name)

(Signature)

(Printed Name)

Date: _____

BID FORM CONTRACT NO. 77-13-01

Ref. No.	Item No.	Item Description	Approx. Quantity	Unit Cost	Unit Cost	Extended Bid Amount
ROADWAY (Ref. Nos. 1 - 55)						
1	SP 201	CLEARING AND GRUBBING, AS PER PLAN	LUMP SUM	-		
2	202	CONCRETE BARRIER REMOVED	608	LIN. FT.		
3	202	PIPE REMOVED, 24" AND UNDER	3,079	LIN. FT.		
4	202	GUARD POST REMOVED	38	EACH		
5	202	HEADWALL REMOVED	16	EACH		
6	202	CATCH BASIN REMOVED	45	EACH		
7	202	PIPE REMOVED OVER 24"	12	LIN. FT.		
8	202	FENCE REMOVED FOR REUSE	190	LIN. FT.		
9	202	GUARDRAIL REMOVED	12,273	LIN. FT.		
10	202	GUARDRAIL REMOVED FOR STORAGE	125	LIN. FT.		
11	202	GUARDRAIL REMOVED FOR REUSE	9,456	LIN. FT.		
12	202	FENCE REMOVED	1,116	LIN. FT.		
13	202	WALK REMOVED	300	SQ. FT.		
14	202	CURB AND GUTTER REMOVED	110	LIN. FT.		
15	202	ANCHOR ASSEMBLY REMOVED, TYPE T	19	EACH		
16	202	ANCHOR ASSEMBLY REMOVED FOR STORAGE	2	EACH		
17	SP 202B	3 CORNER CRACK REPAIR, USING ITEM SP402	10	CU. YD.		
18	SP 202J	PLUG AND FILL EXISTING CONDUIT	555	LIN. FT.		
19	SP 202K	ANCHOR ASSEMBLY, SYRO STEEL ET-2000, REMOVED FOR STORAGE	24	EACH		
20	SP 202L	ANCHOR ASSEMBLY, SYRO STEEL ET-2000, REMOVED AND RESET	2	EACH		
21	203	EXCAVATION NOT INCLUDING EMBANKMENT CONSTRUCTION	62,860	CU. YD.		
22	203	EMBANKMENT	37,569	CU. YD.		
23	204	PROOF ROLLING	44	HOUR		
24	204	SUBGRADE COMPACTION	140,456	SQ. YD.		
25	209	DITCH CLEANOUT, AS PER PLAN	4,881	LIN. FT.		
26	SP 205	LIME MODIFIED SUBGRAOE	66,000	SQ. YD.		
27	SP 205	CEMENT MODIFIED SUBGRADE	66,000	SQ. YD.		
28	SP 205	LIME	3,300	TON		
29	SP 205	CEMENT	4,125	TON		
30	SP 205	WATER	100	M. GAL.		
31	SP 536A	CONCRETE WEATHERPROOFING, MEDIAN WALL/MASONRY COATING	2990	31489	SQ. YD.	
32	604	MONUMENT ASSEMBLY	27	EACH		
33	606	ANCHOR ASSEMBLY, TYPE T	5	EACH		
34	606	BRIDGE TERMINAL ASSEMBLY, TYPE 1, USING STEEL POSTS	24	EACH		
35	606	BRIDGE TERMINAL ASSEMBLY, TYPE 2, USING STEEL POSTS	1	EACH		

INDEX - SPECIAL PROVISIONS

SP 516A	CRACK REPAIR	SP-173
SP 516B	SEALING OF CONSTRUCTION JOINTS	SP-177
SP 519	PATCHING CONCRETE STRUCTURES	SP-179
SP 519A	PATCHING CONCRETE BOX CULVERTS	SP-181
SP 525A	LEAD PAINT REMOVAL - WORKER/ENVIRONMENTAL PROTECTION AND WASTE HANDLING	SP-185
SP 525B	DEMOLITION OF STEEL COATED WITH LEAD PAINT - WORKER/ENVIRONMENTAL PROTECTION/DISPOSAL	SP-217
SP 526	REINFORCED CONCRETE APPROACH SLAB	SP-235
SP 526A	STABILIZE APPROACH SLABS	SP-237
SP 527	FALSEWORK, TEMPORARY BRACING AND PROTECTIVE STRUCTURES	SP-239
SP 528	REPLACE EXISTING RIVET WITH NEW HIGH STRENGTH BOLT	SP-243
SP 533R	REPLACEMENT OF CONTINUOUS STRIP SEAL IN STRUCTURAL STEEL JOINT	SP-245
SP 533W	CONTINUOUS STRIP SEAL IN STRUCTURAL STEEL JOINT (WIDENING)	SP-247
SP 536	CONCRETE WEATHERPROOFING	SP-251
SP 536A	MASONRY COATING	SP-256A-F
SP 604	CATCH BASINS, INLETS, JUNCTION CHAMBERS, AND MANHOLES	SP-257
SP 605	UNDERDRAINS	SP-259
SP 606	GUARDRAIL REBUILT	SP-261
SP 606A	ANCHOR ASSEMBLY, TYRO STEEL ET-2000, OPTION A WITH ALL POSTS IN FOUNDATION TUBES	SP-263
SP 606C	TEMPORARY GUARDRAIL FOR MAINTAINING TRAFFIC	SP-265
SP 606E	ANCHOR ASSEMBLY, TYPE E	SP-269
SP 609	ASPHALT CONCRETE CURB	SP-271
SP 614	MAINTAINING TRAFFIC	SP-273
SP 619	FIELD OFFICE	SP-281
SP 622	PORTABLE CONCRETE BARRIER	SP-283
SP 622C	MEDIAN WALL	SP-285
SP 623	CONSTRUCTION LAYOUT SURVEY	SP-287
SP 625	CONDUIT WITH MULTI-CELL INNERDUCT	SP-289
SP 625A	POLYMER CONCRETE JUNCTION BOX	SP-293
SP 626	RAISED PAVEMENT MARKERS	SP-295
SP 626A	CONSTRUCTION ZONE MARKERS	SP-301
SP 627	STONE SHOULDER PROTECTION	SP-303
SP 641	TEMPORARY PAVEMENT MARKINGS	SP-305
SP 641C	REMOVAL OF PAVEMENT MARKING	SP-309
SP 711	HIGH-STRENGTH STEEL BOLTS, NUTS AND WASHERS	SP-313
SP 730	TRAFFIC CONTROL SIGN AND SUPPORT MATERIAL	SP-315
SP 802	BARRIER REFLECTORS	SP-317
SP 825	GALVANIZED REINFORCING STEEL	SP-321
SP 827B	SPECIAL PROVISIONS FOR PROTECTION OF NORFOLK-SOUTHERN RAILWAY INTEREST	SP-323
SP 827D	SPECIFIC REQUIREMENTS OF CSX TRANSPORTATION	SP-337
SP 853	GROUT ANCHORING WITH NON-SHRINK, NON-METALLIC EPOXY GROUT	SP-339
SP 952	NON-SHRINK, NON-METALLIC EPOXY GROUT	SP-341

SPECIAL PROVISIONS

SP 536A

MASONRY COATING

(10 /12)

A. Description

This Item shall consist of the necessary labor, materials and equipment to clean, prepare and treat Portland cement concrete surfaces, as specified in the Plans or as directed by the Chief Engineer, in accordance with these Specifications as described herein.

B. Materials

The materials shall be Tex-Cote XL-70 Bridge Cote with Silane, manufactured by Textured Coatings of America, Texture DOT with Silane manufactured by Chemmasters, or approved equal and shall be a commercial product designed specifically for coating concrete and shall be suitable for application on damp, uncured concrete and/or cured concrete. The color of the applied masonry coating shall be in accordance with Federal Color Standard No 595 and shall be "Color #36622, Light Gray" or as approved by the Chief Engineer

C. Material Tests and Certification

Prior to the application of any material, the Chief Engineer shall be provided certification confirming that the commercial product furnished is in accordance with the same formula as that previously subjected to tests specified below and approved. Copies of the current test reports shall be attached to the certification. Reports for tests made more than four (4) years prior to the shipment to the Contract will not be accepted. All testing shall be performed by a qualified, commercial testing laboratory, acceptable to the Commission.

The applied finish coat shall be subjected to and shall satisfy the requirements of the tests listed below, prior to the use on this project.

1 FREEZE-THAW TESTS

The applied finish coating shall be subjected to freeze thaw cycle tests as follows:

- a. Three (3) concrete specimens, not less than four inches (4") by six inches (6"), of the mix design for the structure shall be cast and cured Fourteen (14) days moist curing with a drying period at room temperature, sixty degrees (60°) F to eighty degrees (80°) F, for twenty-four (24) hours will be required before the specimens are coated with the applied finish. Caution shall be taken that there be no excessive oil on specimen forms. The finish coating shall be applied to the sides of specimens at a spreading rate of fifty (50) ± ten (10) square feet per gallon. Brush application will be permitted. Cementitious coatings shall be cured at room temperature and thirty (30) percent relative humidity for twenty-four (24) hours, at room temperature and ninety (90) percent relative humidity for forty-eight (48) hours, and at room temperature and fifty (50) percent relative humidity for four (4) days for a total curing time of seven (7) days. Other coatings shall be cured at room temperature for forty (48) hours after the completion of curing

SPECIAL PROVISIONS

- b The specimens shall be immersed in water at room temperature for three (3) hours, and then removed
- c The specimens shall be placed in cold storage at negative fifteen degrees (15°) F for one (1) hour, and then removed.
- d The specimens shall be thawed at room temperature for one (1) hour
- e Steps c and d shall be repeated for a total of 250 cycles. At the end of 250 cycles, the specimens shall show no visible defects

2 ACCELERATED WEATHERING

The applied finish coating shall be subjected to a 7,500 hour exposure test in a Twin-Carbon-Arc-Weatherometer, ASTM G 23, Type D, at an operating temperature of 145 degrees F. The test shall be made at twenty (20) minute cycles consisting of seventeen (17) minutes of light and three (3) minutes of water spray plus light. At the end of the exposure test, the exposed samples shall show no chipping, flaking, or peeling. The panels for this test shall be prepared by applying the coating at a spreading rate of fifty (50) + ten (10) square feet per gallon to both sides and edges of panels cut from asbestos cement shingles in accordance with Federal Specifications SS-S-346, Type I. Curing time shall be in accordance with Section 1.a.

3 FUNGUS GROWTH RESISTANCE

The applied finish coating to be used shall pass a fungus resistance test in accordance with Federal Specification TT-29g. Fungus growth shall be indicated after a minimum incubation period of twenty-one (21) days

4 ABRASION RESISTANCE

The applied finish coating to be used shall pass the 2,000 liter sand abrasion test in accordance with Method 6191 Abrasion Resistance — Falling Sand, Federal Test Method Standard 141a, ASTM D968-81. The specimens for this test shall be prepared by applying the coating to a cleaned steel panel at a spread rate of fifty (50) + ten (10) square feet per gallon. The specimens shall be cured at room temperature for twenty-one (21) days.

5 IMPACT RESISTANCE

The finish coating shall be applied to a concrete panel prepared according to Federal Test Method Standard 141a, Method 2051, at a spreading rate of fifty (50) + ten (10) square feet per gallon, and allowed to cure for twenty-one (21) days at room temperature. The test shall be run using the Gardner Mandrel Impact Tester in accordance with ASTM D 2794 using a one-half inch (1/2") indenter load of twenty-four (24) inch-pounds. The coating shall show no chipping under this impact load.

6 SALT-SPRAY RESISTANCE TEST

A concrete specimen shall be coated at the rate of one (1) gallon per fifty (50) + ten (10) square feet and cured for twenty-one (21) days at room temperature. The coated specimen shall be exposed to a five (5) percent salt solution in accordance with ASTM B117 for 2,500 hours.

SPECIAL PROVISIONS

where the atmospheric temperature is maintained at ninety degrees (90°) F + two degrees (2°) F At the end of 2,500 hours of exposure, the coating shall show no ill effects, loss of adhesion, or deterioration.

7 FLEXIBILITY TEST

A sheet metal specimen shall be coated with the applied finish coating at a rate of one (1) gallon per forty-five (45) square feet + ten (10) percent and allowed to cure for forty-eight (48) hours at room temperature. The coated specimen shall be bent 180 degrees over a one inch (1") round mandrel. After bending, the coating shall show no breaking.

In addition to the certification and test reports required above, a service record shall be supplied showing that the finish coating material has a satisfactory service record on concrete surfaces for a period of not less than five (5) years prior to the date of submission of the service record. The finish coating shall also have shown satisfactory service characteristics without peeling, chipping, flaking, and non-uniform change in texture or color. A specific structure for the specific product shall be named for the service record. In addition to the above requirements, the manufacturer shall submit, for each batch of material used, the following product analysis data:

- a. Weight per gallon.
- b. Viscosity in Kreb units
- c. Weight percent pigment
- d. Weight percent vehicle solids.
- e. Infrared spectra of vehicle solution

D. Manufacturer's Representative

The Contractor shall give the manufacturer's technical representative notice of the intended date of application. The Contractor shall have the manufacturer's representative present during the testing and initial installation

E. Surface Condition and Preparation

1. Surface preparation prior to the application of an applied finish coating shall consist of a general surface finish in accordance with the specified requirements. Cavities, which require grout filling, shall have been filled prior to coating application. Air pockets that are one-quarter inch (1/4") in width and depth or less need not be grouted prior to application of the finishing coat. Air pockets that are larger than one-quarter inch (1/4") in width and depth shall be filled with grout consisting of one (1) part Portland cement, two (2) parts screened and washed sand graded to pass the sixteen (16) mesh sieve with no more than five (5)% retained on the thirty (30) mesh sieve and sufficient water to produce a thick liquid mix, or by the use of a grout composed of the same materials used for the applied finish coating. The grout shall be applied filling the air pockets by using burlap pads, float sponges or other acceptable methods. As soon as the grout has taken its initial set, the surface shall be brushed to remove all loose grout, leaving the surface smooth and free of any air holes
2. Surfaces to which coatings are to be applied shall be free from efflorescence, flaking coatings, dust, dirt, oil, wax, curing compounds, laitance, foreign materials, deleterious substances and shall be

SPECIAL PROVISIONS

structurally sound. Weak sections and spalled areas shall have been repaired before application of the coating. Curing compound and form release agent must be removed and may require light sandblast or waterblast at a minimum 2,500 p s i or greater

3. The use of chemicals and other cleaning compounds to facilitate the removal of these foreign materials shall be approved by the coating manufacturer or its representative before use
4. Prior to the application of the finish coating, the surfaces shall have been prepared in accordance with the manufacturer's recommendations and shall be in a condition consistent with the manufacturer' requirements.

F. Test Application

Prior to the final application, the Contractor shall apply masonry coating to measured test coverage areas on horizontal and vertical surfaces of the different components of the structure to be coated for the purpose of demonstrating the desired physical and visual effect of masonry coating application necessary to achieve the specified coverage rate as per the manufacturer's recommendations. In the latter case, the applicator shall use at least one-half (1/2) gallon of masonry coating following the manufacturer's recommended method of application for the total of the test surfaces.

G. Application

1. The application, including equipment used, shall be in accordance with the manufacturer's recommendations. The material shall be applied by qualified personnel experienced in the Work
2. When used as a curing compound, curing shall be performed in accordance with Section 511 17 Method (B) of the Specifications and at an application rate specified by the manufacturer to achieve an adequate curing membrane, uniform appearance, texture and color, as approved by the Chief Engineer.
3. The coating material shall have a shelf life of not less than twelve (12) months. The material shall be delivered to the job site in sealed containers bearing the manufacturer's original labels. The brand, color and type shall be clearly marked on each container. A copy of the manufacturer's printed instructions shall be provided to the Chief Engineer
4. The coating material shall be stored prior to surface preparation and application of the material in airtight, upright containers. The containers shall be stored in a dry location where the temperature is above forty degrees (40°) F and less than 100 degrees F
5. The material shall be thoroughly mixed in its original container. If skins have formed, they shall be removed prior to mixing the material. The material shall not be thinned
6. The material shall be applied at a uniform film thickness at a rate of fifty (50) + ten (10) square feet per gallon. The application rate shall be sufficient enough to produce a uniform color and texture

SPECIAL PROVISIONS

7. The material shall be applied only when the ambient temperature is between forty degrees (40°) F and rising, 100 degrees F
8. The material shall be capable of being applied on damp, but not wet concrete surfaces. The material shall not be applied over frozen surfaces or if rain is imminent. Should rain occur on a freshly applied surface, re-coating may be necessary, at the Chief Engineer's discretion
9. Every attempt should be made to schedule the application of the masonry coating as one (1) of the final operations to minimize construction-generated dust
10. To prevent lap marks a wet edge shall be maintained at all times. Stopping and starting at mid-sections shall be avoided. Every attempt shall be made to start or end at natural breaks in the surface such as at a panel edge, corner or joint. When applying the coating with a roller, the material shall be applied in vertical strokes initially, cross rolled for even film and appearance, and then finished with vertical strokes
11. Only one (1) coating material shall be used on an individual structure.
12. After application, the coating shall be dry to the touch within forty-eight (48) hours and shall achieve a final cure within two (2) to three (3) weeks under ideal conditions

H Finish Product and Appearance

The coating material in the finished state shall be capable of accommodating the thermal and elastic expansion ranges of the substrate without cracking.

The texture of the completed finish coat shall be generally similar to that of rubbed concrete. The completed finished coating shall be tightly bonded to the structure and present a uniform appearance and texture. If necessary, additional coats shall be applied to produce the desired surface texture uniformity.

Coatings shall be entirely removed from the structure upon its failure to positively adhere without chipping, flaking or peeling, or attaining the desired surface appearance. The finish coating shall be reapplied after proper surface preparation until the desired finish product is achieved. The average thickness of the completed finish coating shall not exceed 1/8 of an inch.

I. Precautions

Precautions shall be followed as indicated on the manufacturer's MSDS

J Protection of Adjoining Surfaces and The Public

When applying a masonry coating, the Contractor shall protect, by masking off or by other means, adjoining surfaces of the structure, which are not to be coated. The Contractor shall also make provisions to protect the public when applying masonry coating to the fascia of a bridge and/or any portions of the structure directly adjacent to the traveling public. The Contractor shall also ensure that when using spraying as an application method that all overspray is contained within the operation itself and does in no way cause exposure to the traveling public.

SPECIAL PROVISIONS

K. Environmental Requirements

Protect plants and vegetation from overspray by covering with drop cloths

L. Measurement of Payment

Only those measurements necessary to verify application rates will be made. Payment shall be based on the actual area in square yards of the coated surface and shall include surface preparation, material, application costs and necessary labor and incidentals for the Work

Payment shall be made under:

<u>Item</u>	<u>Unit</u>	<u>Description</u>
SP 536A	Sq. Yd	Masonry Coating

SPECIAL PROVISIONS

SP 622

PORTABLE CONCRETE BARRIER

(11-21-12)

A. Description

This Item shall consist of furnishing, installing and resetting as required, portable concrete barrier and for maintaining traffic in accordance with this Special Provision and as shown on the Plans. All applicable provisions of Section 622 of the Specifications, SP 104 – Access to Turnpike and Restrictions and SP 614 – Maintaining Traffic, of these Special Provisions shall apply

This Item shall also include maintaining, moving, transporting and storing of the portable barriers for the duration of the Contract. Upon completion of the Contract, the barriers shall remain the property of the Contractor and shall be removed from the Turnpike right-of-way.

Portable concrete barriers furnished shall be in accordance with ODOT Standard Drawing RM-4.2, or other shape approved by the Chief Engineer. Bridge mounted portable barrier shall be in accordance with ODOT Standard Drawing PCB-91. The barrier must be NCHRP 350, Test Level 3 compliant.

The portable barrier shall have new barrier delineators attached to the traffic side of the barrier at one (1) per each barrier section. And the top of the delineator shall be located just below the angle of the barrier face (approximately nineteen inches (19") below the top of the barrier). The delineators shall be oriented so as to face oncoming traffic and shall be in place before the barrier is exposed to traffic. Each delineator shall be installed with a slight backward slant so that the reflective surface is exposed to rain to help keep the delineators clean. The delineators shall be attached with adhesive after the concrete barrier has been cleaned with a steel wire brush to remove loose concrete and/or dirt.

B. Materials

~~Portable concrete barrier shall be as specified in Section 622.02 of the Specifications.~~

Portable Concrete Barrier Option 1: Portable concrete barrier in accordance with this section, ODOT CMS 622 Portable Concrete Barrier and ODOT Standard Drawings RM-4.3 32" Portable Concrete Barrier.

Portable Concrete Barrier Option 2: A portable steel barrier system comprised of nominally eight (8) gauge thick ASTM A36 pressed steel panels, galvanized prior to assembly. Barrier sections shall be 50 feet long and 32 inches in overall height. Base width shall be a minimum of 2.3 feet and a top width of 6.25 inches. The barrier shall have a mountable foot section and have rubber feet fastened to the sections at 4 foot intervals. A lifting system shall be incorporated into the sections. Installation of the sections shall not impede drainage. Each section shall be anchored to the pavement at a minimum of two points. Portable steel barrier shall meet the

SPECIAL PROVISIONS

requirements of NCHRP Report 350 Test Level 3 (TL3) and Test Level 4 (TL4).

Barrier delineators shall be "Astro Optics Corporation Reflective Delineators Series JD-1, Amber "

Adhesive for barrier delineators shall be "Signal Products Epoxy Adhesive" or approved equal.

C Construction Requirements

Single lane traffic zones shall be permitted on both the Eastbound and Westbound lanes during the removal and resetting of the portable concrete barrier in accordance with the provisions of SP 104 – Access to Turnpike and Restrictions.

Any portable concrete barriers damaged during the Contract shall be promptly replaced by the Contractor at no additional cost to the Commission. Damaged sections shall be disposed of in accordance with SP 105 – Disposal of Excess Materials.

Barrier delineators shall be cleaned on a monthly basis or as directed by the Chief Engineer to provide maximum effectiveness of the reflectorization

D. Method of Measurement

Portable concrete barrier shall be paid for as lump sum. The lump sum price bid shall include all barrier delineators, including replacement delineators during construction, as well as the delivery, installation, maintaining, cleaning delineators, moving, transporting, storing and re-setting, as required, of the barriers throughout the term of the Contract. Bridge mounted portable barrier shall also include providing and installing anchors, removal of the anchors, patching of the bridge deck, and weatherproofing the patch area per SP 536.

E. Basis of Payment

Payment shall be made at the lump sum price bid for:

Item	Unit	Description
SP 622	Lump Sum	Portable Concrete Barrier (Furnished by Contractor)
SP 622	Lump Sum	Portable Concrete Barrier, Bridge Mounted (Furnished by Contractor)

INDEX OF SHEETS

1. TITLE SHEET	2,3
SURVEY CONTROL PLAN	4
SCHEMATIC PLANS	5-10
SECTION LOCATIONS	11-13, 13A, 13B, 13C, 14, 15
GENERAL NOTES	16-32, 33-35, 35A, 35B, 35D
Maintenance of Traffic—Mainline	36-41
Maintenance of Traffic—Sideroads	42-44
CALCULATIONS & SUBSUMMARIES	45-59
STORM WATER POLLUTION PREVENTION PLAN	60-70
PLAN & PROFILE	71-93
PAVEMENT ELEVATION TABLE	94-114
MEDIAN WALL PLAN AND ELEVATION	115-118
CROSS SECTIONS	119-122
DRAINAGE DETAILS	123-124
APPROACH SLAB DETAILS	125-126
TRAFFIC CONTROL	206-223
STRUCTURE GENERAL NOTES & COMMON DETAILS	224-234
BRIDGE OVER: U.S. 20 (MP 59.8)	235-246
BRIDGE OVER: MICHIGAN Ave. (MP 61.1)	247-262
BRIDGE OVER: N. & S. R. & STENGEL Ave. (MP 61.5)	263-279
BRIDGE OVER: S.R. 65 (MP 63.3)	278-289
BRIDGE OVER: CSX RR.	290-300
BRIDGE OVER: WHITE RD. (MP 63.6)	301-311
BRIDGE OVER: SIMONS RD. (MP 63.9)	312-322
TERRA TECH CROSSMAN DITCH MODIFICATIONS	1-9
SOLI PROFILE/SUBSURFACE INVESTIGATION PLANS	



OHIO TURNPIKE COMMISSION

THE JAMES W. SHOCKNESSY OHIO TURNPIKE

CONTRACT NO. 77-13-01
THIRD LANE CONSTRUCTION
M.P. 59.52 TO M.P. 64.13

STATION 586+50.00 TO 772+56.56, LUCAS COUNTY
STATION 0+00.00 TO 59+00.00, WOOD COUNTY

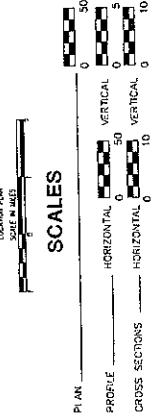
OHIO TURNPIKE COMMISSION STANDARD DRAWINGS

	BEGIN PROJECT STA.	END PROJECT STA.	APPROVED FOR THE OHIO TURNPIKE COMMISSION BY John P. Stengel
AS-1	CBR-1	06-25-07	01-24-11
AS-2	CBR-1	06-25-07	01-24-11
AS-3	CBR-2	06-25-07	01-24-11
AS-4	CBR-3	06-25-07	01-24-11
AS-5	CBR-4	06-25-07	01-24-11
CB-1	CBR-5	06-25-07	01-24-11
CB-2	CBR-5	06-25-07	01-24-11
CB-3	CU-1	06-25-07	01-24-11
CB-4	CU-1	06-25-07	01-24-11
CB-5	CU-2	06-25-07	01-24-11
CBM-1	DU-1	06-25-07	01-24-11
CBM-2	DU-2	06-25-07	01-24-11
CBM-3	DR-1	06-25-07	01-24-11
CBM-4	EPA-1	06-25-07	01-24-11
CBM-5	GR-1	06-25-07	01-24-11
CBM-6	GR-2	06-25-07	01-24-11
CBM-7	JB-1	06-25-07	01-24-11
CBM-8	MC-C-1	06-25-07	01-24-11

UNDERGROUND UTILITIES
CONTACT BOTH SERVICES
CALL TWO WORKING DAYS
BEFORE YOU DIG

FIBER OPTIC CABLE AS-BUILT DRAWINGS

	PLAN	PROFILE	CROSS SECTIONS
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			

SCALES


PLANS PREPARED BY
DANSHARD GROHNE LONG, Limited

Toledo, Ohio

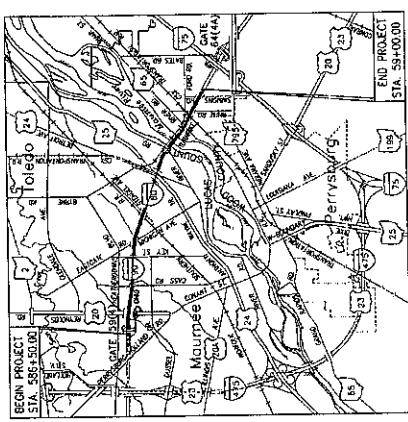
REVIEW CONSULTANT
SRS Corporation

ADDENDUM 1 12/10/12
REVIEW DATE
NOVEMBER 14, 2012

DATE
11/26/2012

DATE
12/10/12

DESIGN CONTRACT NO. 77-9715



	PLAN	PROFILE	CROSS SECTIONS
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			

OHIO DEPARTMENT OF TRANSPORTATION STANDARD DRAWINGS

	PLAN	PROFILE	CROSS SECTIONS
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			
1. 2. 20-34. 60-71.			

PLANS PREPARED BY
DANSHARD GROHNE LONG, Limited

Toledo, Ohio

REVIEW CONSULTANT
SRS Corporation

ADDENDUM 1 12/10/12
REVIEW DATE
NOVEMBER 14, 2012

DATE
11/26/2012

DATE
12/10/12

DESIGN CONTRACT NO. 77-9715

SEQUENCE OF CONSTRUCTION
THESE PLANS ARE BASED ON THE FOLLOWING SUGGESTED SEQUENCE OF CONSTRUCTION. THE CONTRACTOR MAY SUBMIT AN ALTERNATE SEQUENCE OF CONSTRUCTION FOR APPROVAL. NO ALTERNATE SEQUENCE OF CONSTRUCTION SHALL BE IMPLEMENTED WITHOUT WRITTEN APPROVAL OF THE CHIEF ENGINEER.

THE ADDITION OF A THIRD DIRECTIONAL LANE AND A BARRIER WALL MEDIAN ALONG THE OHIO TURNPIKE SHALL BE ACCOMPLISHED WHILE TRAFFIC IS MAINTAINED IN ACCORDANCE WITH THE ENCLOSED MAINTENANCE OF TRAFFIC PLANS.
THE ROADWAY CONSTRUCTION CONTRACT WILL INVOLVE A SHARING OF COMMON LANE AREAS DURING PHASE ONE WITH ANOTHER ADDING CONSTRUCTION CONTRACT. COOPERATION BETWEEN CONTRACTORS IS VITAL TO THE SUCCESS OF THE TOTAL PROJECT. THE CONTRACTOR SHALL SUBMIT A PLAN AND SCHEDULE OF OPERATIONS TO THE CHIEF TURNPIKE CONSTRUCTION MANAGEMENT AFTER APPROVAL.

PHASE ONE CONSTRUCTION (2013)
THE EXISTING OUTSIDE SHOULDER IS TO BE MILLED AND REINFORCED. SEE THE ROADWAY CONSTRUCTION PLANS FOR SHOULDER BUILD-UP AND WORK ZONE FRENZONING. THE OUTSIDE DRAINAGE WORK (SOFT DRAIN AND AS-REMOVED DRAIN) SHALL BE PERFORMED BY THE CONTRACTOR. THE STATE ROUTE 65 BRIDGE OVER THE TURNPIKE WILL BE MOVED AND THE NEW BRIDGE AND CATCH BASIN AND SOFT DRAIN REPLACEMENT SHALL BE COMPLETED. THIS CONSTRUCTION EFFORT SHOULD HAVE A SHORT TIME-FRAME AND IS COVERED IN SPECIAL PROVISION SP 104.

PHASE ONE MAINTENANCE OF TRAFFIC (2013)
INSTALL LAST-BOUND LEAD-IN SIGNING RELATIVE TO REDUCING THE EXISTING TWO-DIRECTION LANE TO ONE. INSTALL METABOND LEAD-IN SIGNING RELATED TO REDUCING THE EXISTING THREE-DIRECTION LANE TO ONE.

PLACE THE MAINTENANCE OF TRAFFIC DEVICES THROUGH THE TAPER AREA AND ALIGN THE MARGIN IN ACCORDANCE WITH THE PHASE ONE MAINTENANCE OF TRAFFIC TYPICAL SECTION AND PLANS.

PHASE TWO CONSTRUCTION (2013–2014)
THE MAJOR ELEMENT OF THIS PHASE IS THE CONSTRUCTION OF THE TURNPIKE MEDIAN WHICH INCLUDES AN ADDITIONAL DIRECTIONAL LANE, A NEWED SHOULDER AND A MEDIAN BARRIER SAFETY WALL. ALL EXISTING MARSHAL BRIDGES ARE TO BE REMOVED.

PHASE TWO MAINTENANCE OF TRAFFIC (2013–2014)
TWO LANES OF TRAFFIC ARE TO BE USED ALONG THE TURNPIKE USING TWO LANE AND RECONSTRUCTED OUTSIDE SHOULDER. SEE PHASE TWO MAINTENANCE OF TRAFFIC TYPICAL SECTION. PRIOR TO THE WORK IN THE TAPER AREA, THE OUTSIDE SHOULDER SHALL BE MOVED AS SHOWN IN THE PLANS. BLOCKS WILL BE MOVED TO USE THE LEFT LANE. COUNTS OF 6 CARS PER MINUTE WILL BE MAINTAINED. THE OUTSIDE SHOULDER AND BARRIERS SHALL BE RELOCATED AND BUILT AS SHOWN IN THE PLANS. SETTING THE TEMPORARY CONCRETE BARRIER SHALL BE AS PER TUP-1.

FOR INSTALLATION OF THE BRIDGE EXPANSION JOINT STRIP SEAL, THE CONTRACTOR HAS THREE LINES OF PAYMENT AND BOTH SHOULDER PLACEMENTS. TO PROVIDE SHORT TERM SHIFT USE, USE OHIO TURNPIKE COMMISSION STANDARD DRAWING TCR-17 FOR SIGN AND DRAWS.

OPTIONAL WHITE PHASE CONSTRUCTION

THE CONTRACTOR SHALL BEGIN THE MAINLINE BRIDGE PIP AND THE OUTLINE CONSTRUCTION IN THE MEDIAN. NO EXISTING BRIDGE PARAPETS ARE INTENDED TO BE REACHED.

OPTIONAL WHITE PHASE MAINTENANCE OF TRAFFIC (2013–2014)

TWO DIRECTIONAL LANES SHALL BE MAINTAINED AT ALL TIMES UNLESS GRANTED BY THE CHIEF TURNPIKE COMMISSION, CHIEF ENGINEER. ANY CHANGE BEYOND THE ENCLOSED TEMPORARY TRAFFIC CONTROL PLANS SHALL BE MADE IN WRITING. THE CONTRACTOR SHALL NOTIFY THE CHIEF TURNPIKE COMMISSION, CHIEF ENGINEER. THE CONTRACTOR SHALL SUBMIT A DETAILED REQUEST FOR CHANGE A MINIMUM OF 10 DAYS PRIOR TO ALLOW DR REVIEW TIME.

THE MAINLINE BRIDGE, INTEGRIC AT THE CSX RAILROAD (MP 61.5) AND THE NORTHERN SOUTHERN RAILROAD (MP 61.5) SHALL NOT BEGIN UNTIL THE SECOND YEAR OF CONSTRUCTION (2014).

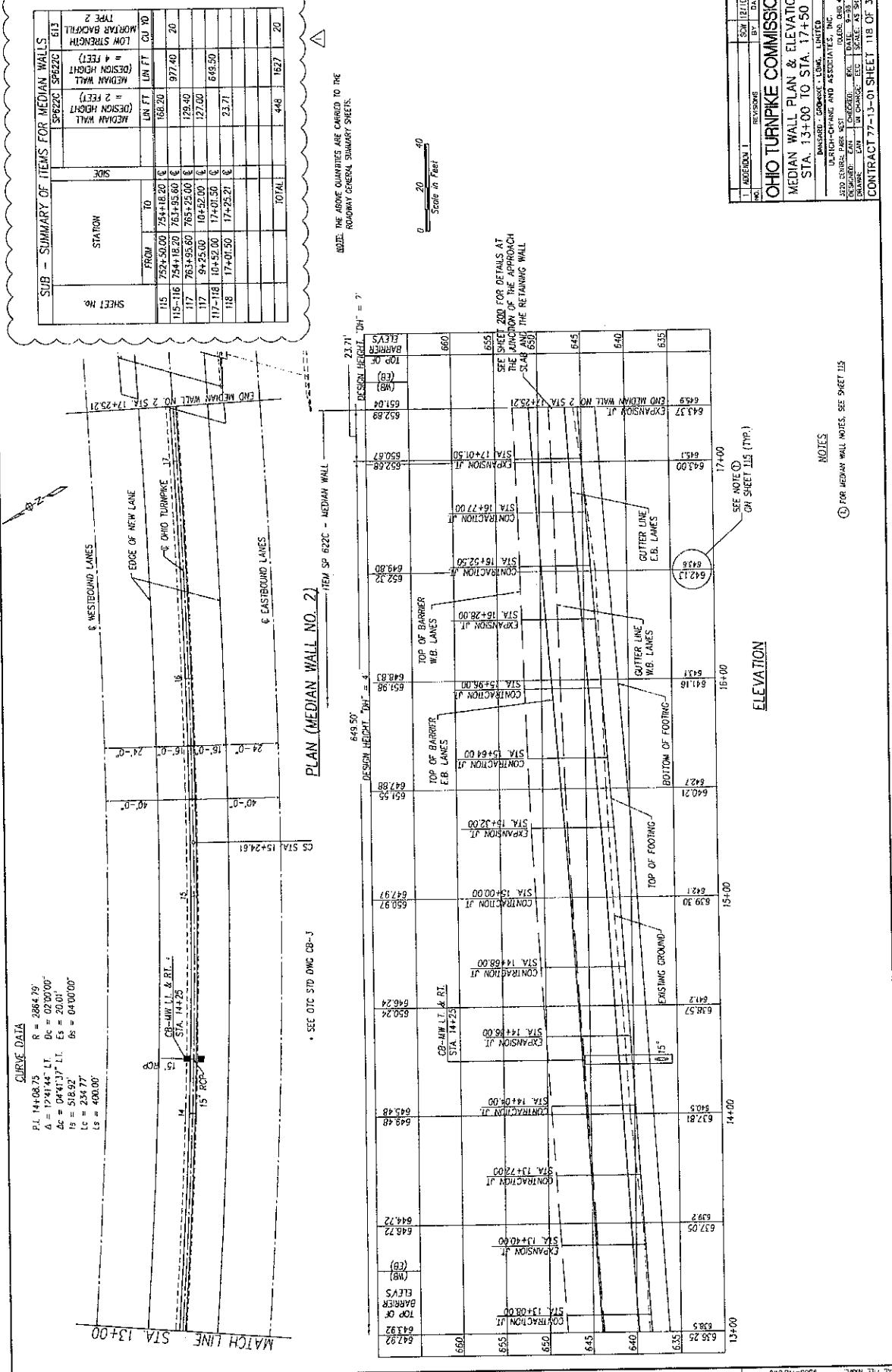
ITEM NUMBERS	LOCATION		STATION TO STATION		DIRECTION		AS PER PLAN	IMPACT ATTENUATOR	CONCRETE BARRIER DELINEATOR (FOR INTERMEDIATE BARRIER ONLY)	TEMPORARY WHITE EDGE LINE	ZONE FRENZON	TEMPORARY WHITE CHANNELIZING LINE	REMOVAL OF PARAPET/MARBLING	BARRIER REFLECTOR TYPE A	BARRIER REFLECTOR TYPE B	
	SP 606	SP 622	SP 626	SP 641	SP 641 SP 645	SP 652										
2013 CONSTRUCTION																
- MP 59.70 TO MP 60.44	EB															
- MP 59.00 TO MP 60.58	WB															
2014 CONSTRUCTION																
2 MP 59.28 TO MP 60.36	EB	1	29.78	5.64	537	5.98	5.08	0.30	0.15	10.16						
2 MP 59.24 TO MP 60.48	WB	1	29.78	5.64	536	5.64	5.64	0.30	0.15	11.28						
CONSTRUCTION ACCESS AND STRUCTURES																
2 MP 60.53 ACCESS	EB	2														
2 MP 62.58 ACCESS	WB	2														
2 MP 59.86 US 20 BRIDGE	WB	2														
2 MP 61.10 MICHIGAN AVENUE BRIDGE	WB	2														
2 MP 61.58 NORFOLK SOUTHERN RR BRIDGE	WB	2														
2 MP 63.0*	EB	1														
2 MP 63.38*	EB	1														
2 MP 63.7*	EB	1														
2 MP 63.08*	WB	1														
2 MP 63.42*	WB	1														
2 MP 63.58*	WB	1														
2 MP 63.70*	WB	1														

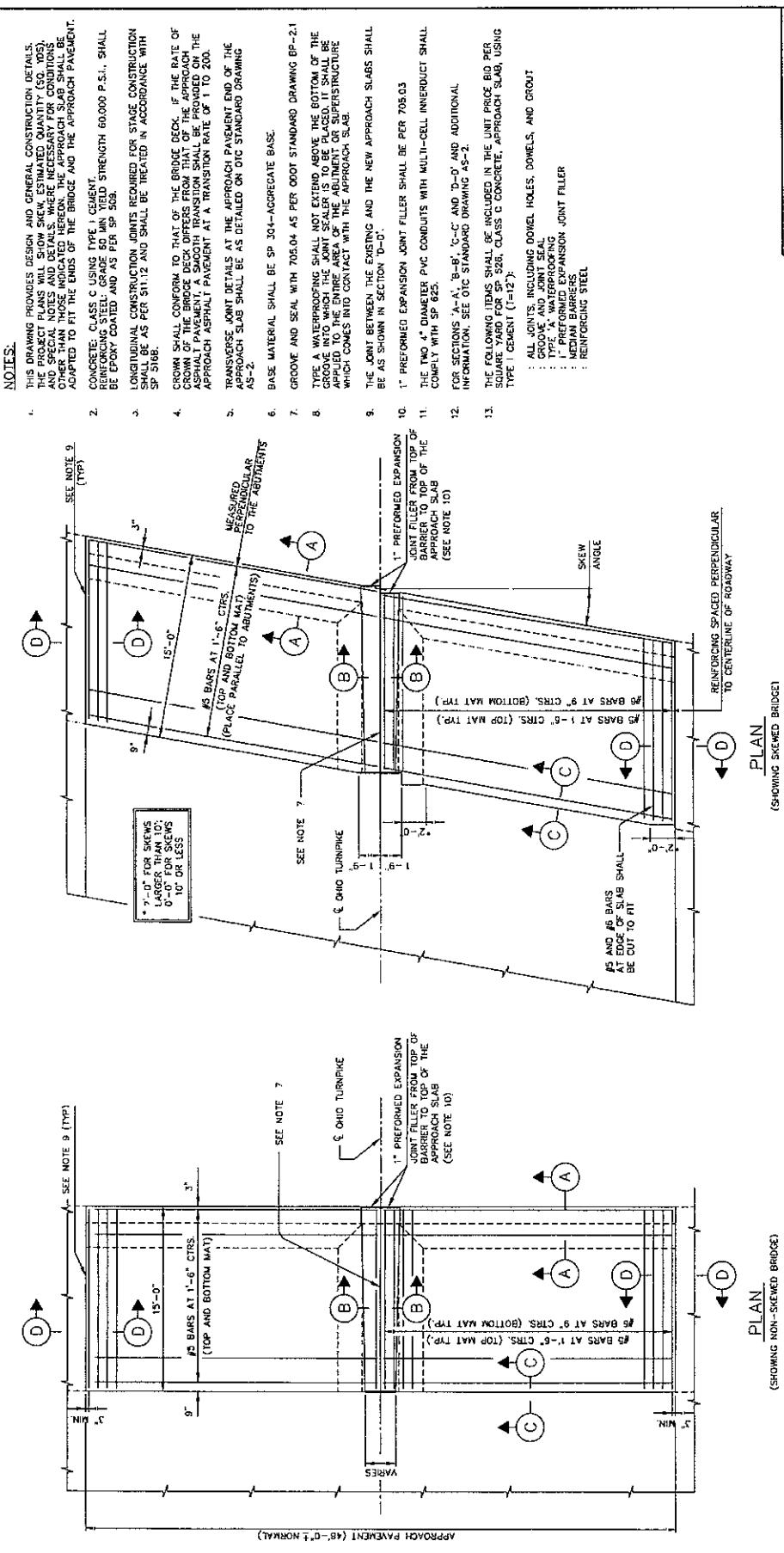
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- SEE SHEET 135A TO 35D														

1 ADDRESS 1	SCW	12/10/12
NO	REVISION	DATE
OHIO TURNPIKE COMMISSION		
SEQUENCE OF CONSTRUCTION AND QUANTITY SUBSUMMARY		

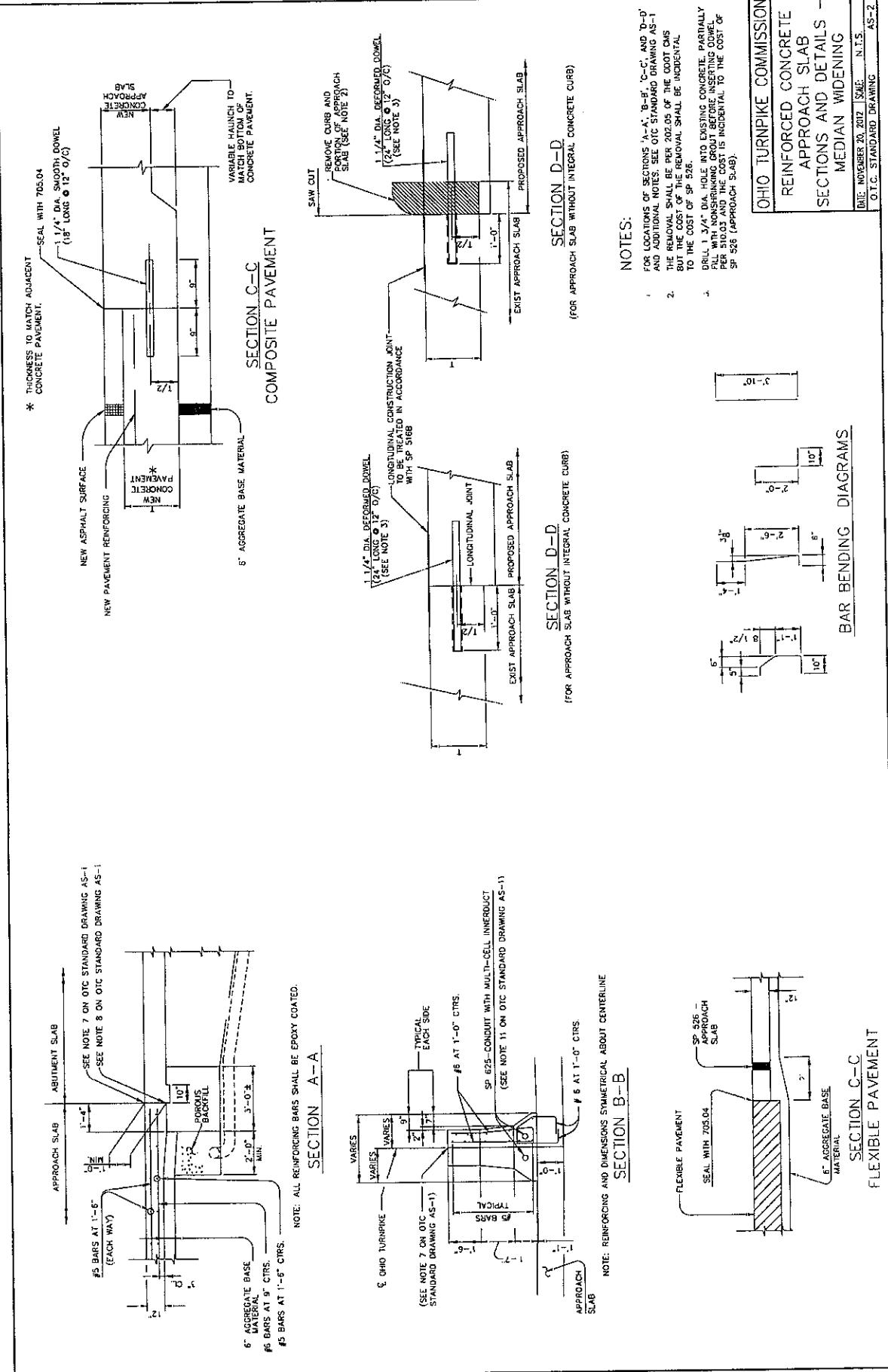
DATE: SEPTEMBER 2012	SCW:	17	Q: 322
CONTRACT: 77-13-01	SHEET:	17	Q: 322

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OHIO TURNPIKE COMMISSION
REINFORCED CONCRETE
APPROACH SLAB —
MEDIAN WIDENING
DATE NUMBER 20-302 SCALE N.T.S. AS-1
O.T.C. STANDARD DRAWING



NOTES:

THIS DRAWING PROVIDES DESIGN AND GENERAL CONSTRUCTION DETAILS. THE PROJECT PLANS WILL SHOW CURBS, CUBES, AND DETAILS WHERE QUANTITY (SQ. YDS.) AND SPECIAL NOTES AND DETAILS, WHERE APPROACH SLAB SHALL BE ADJUSTED TO FIT THE ENDS OF THE BRIDGE, THE APPROACH PAVEMENT.

CONCRETE, CLASS C USING TYPE I CEMENT, STRENGTH 60 UNI YIELD 60,000 P.S.I., SHALL BE EPOXY COATED AND AS PER SP 505.

LONGITUDINAL CONSTRUCTION JOINTS REQUIRED FOR STAGE CONSTRUCTION WITH SP 516B.

CROWN, SHALL CONFORM TO THAT OF THE APPROACH PAVEMENT AND BRIDGE DECK IF THE RATE OF CROWN OF THE BRIDGE DECK DIFFERS FROM THAT OF THE APPROACH PAVEMENT. A SMOOTH TRANSITION SHALL BE PROVIDED ON THE APPROACH PAVEMENT AT A TRANSITION RATE OF 1 TO 200.

TRANSVERSE JOINT DETAILS AT THE APPROACH PAVEMENT END OF THE APPROACH SLAB SHALL BE AS DEFINED ON STANDARD DRAWING AS-5.

BASE MATERIAL SHALL BE SP 304-AGGREGATE BASE.

GROOVE AND SEAL WITH 705.04 AS PER 900 STANDARD DRAWING BP-2.1.

TYPE A WATERPROOFING SHALL NOT EXTEND ABOVE THE BOTTOM OF THE GROOVE INTO WHICH THE JOINT SEALER IS TO BE PLACED. IT SHALL BE APPLIED TO THE ENTIRE AREA OF THE ADJUNCT OR SUPERSTRUCTURE WHICH COMES INTO CONTACT WITH THE APPROACH SLAB.

THE INTERNAL CONCRETE CURB SHALL BE PROVIDED ON THE NEW APPROACH SLAB AS INDICATED ON THE PROJECT PLANS AND SHOWN IN SECTION D-C.

1" PERFORATED EXPANSION JOINT FILLER SHALL BE PER 705.03.

6. BASE MATERIAL SHALL BE SP 304-AGGREGATE BASE.

7. GROOVE AND SEAL WITH 705.04 AS PER 900 STANDARD DRAWING BP-2.1.

8. TYPE A WATERPROOFING SHALL NOT EXTEND ABOVE THE BOTTOM OF THE GROOVE INTO WHICH THE JOINT SEALER IS TO BE PLACED. IT SHALL BE APPLIED TO THE ENTIRE AREA OF THE ADJUNCT OR SUPERSTRUCTURE WHICH COMES INTO CONTACT WITH THE APPROACH SLAB.

9. THE INTERNAL CONCRETE CURB SHALL BE PROVIDED ON THE NEW APPROACH SLAB AS INDICATED ON THE PROJECT PLANS AND SHOWN IN SECTION D-C.

10. 1" PERFORATED EXPANSION JOINT FILLER SHALL BE PER 705.03.

11. CURBS, BRIDGES WITH SIDEWALKS, FOR BRIDGES, CONSTRUCTED WITH RAISED SIDEWALLS, DEFLECTOR PAVES, OR OTHER TYPES OF SIDEWALLS, SHALL BE TIED IN RELATION TO THE SURFACE OF THE APPROACH PAVEMENT. THEY SHALL BE CONSTRUCTED IN COMBINATION WITH OTHER TYPES OF SIDEWALLS, AS DETERMINED BY THE ENGINEER.

12. BRIDGE CURBS, CURB HEIGHT SHALL BE TRANSITIONED UNIFORMLY BETWEEN BRIDGE CURB HEIGHT AND APPROACH CURB HEIGHT IN LENGTH AS FOLLOWS: WHERE WALKWAY EXTENDS BEYOND END OF APPROACH, THE APPROACH SLAB LENGTH SHALL BE ADJUSTED SO AS TO PROVIDE A SMOOTH TRANSITION. HOWEVER, THE TRANSITION LENGTH SHALL NOT BE LESS THAN 10 FT. AND THE TRANSITION SHALL EXTEND BEYOND THE END OF THE APPROACH SLAB IF NECESSARY. CURB PLACEMENT SHALL BE IN ACCORDANCE WITH 900 STANDARD DRAWING BP-1.

13. APPROACH SLAB WITH SHALLOW EXTEND FROM CURTER LINE TO CURTER LINE AND BE 6" WIDER FOR EACH CURB BEYOND THE END OF THE PARAPETS.

14. THE FOLLOWING ITEMS SHALL BE INCLUDED IN THE UNIT PRICE AND PER SQ. YARD FOR SP 522, CLASS C CONCRETE APPROACH SLAB, USING TYPE I CEMENT ("1-2"):

A. ALL JOINTS, INCLUDING DOWEL HOLES, DOWELS, AND GROUT

B. GROOVE AND JOINT SEAL

C. TYPE A WATERPROOFING

D. PERFORATED EXPANSION JOINT FILLER

E. REINFORCING STEEL

15. SEE SECTION "A-A", "C-C" AND "D-D" AND ADDITIONAL INFORMATION SEE 900 STANDARD DRAWING AS-5.

16. APPROACH SLAB, CENTERLINE OF ROADWAY

17. APPROACH SLAB, CENTERLINE OF ROADWAY MWT

18. APPROACH SLAB, PLAN (SHOWING SKEWED BRIDGE)

19. APPROACH SLAB, PLAN (SHOWING NON-SKewed BRIDGE)

20. APPROACH SLAB, PLAN (SHOWING NON-SKewed BRIDGE)

21. APPROACH SLAB, PLAN (SHOWING NON-SKewed BRIDGE)

22. APPROACH SLAB, PLAN (SHOWING NON-SKewed BRIDGE)

REINFORCED CONCRETE
APPROACH SLAB -
CELLULAR ABUTMENTS

DATE: NOVEMBER 30 2012

SCALE: N.I.S.

AS-3

G.I.C.: STANDARD DRAWING

OHIO TURNPIKE COMMISSION

DATE: NOVEMBER 30 2012

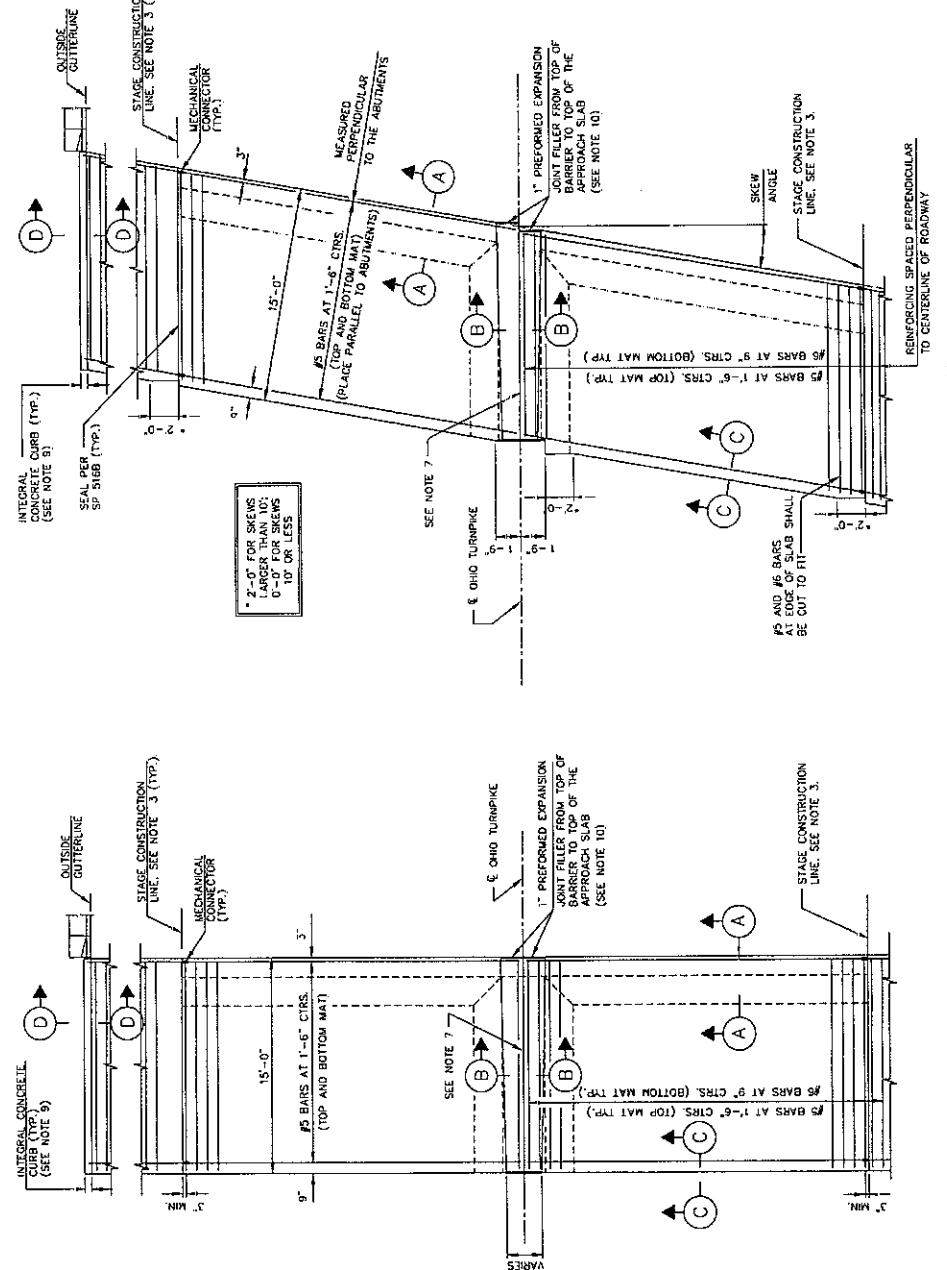
SCALE: N.I.S.

AS-3

G.I.C.: STANDARD DRAWING

NOTES:

- 1. THIS DRAWING PROVIDES DESIGN AND GENERAL CONSTRUCTION DETAILS. THE PROJECT PLANS WILL SHOW SKW. ESTIMATED QUANTITY (SQ. YDS), AND SPECIAL NOTES AND DETAILS WHERE NECESSARY FOR CONDITIONS OTHER THAN THOSE INDICATED HERON. THE APPROACH SLAB SHALL BE ADDED TO FIT THE ENDS OF THE BRIDGE AND THE APPROACH PAVEMENT.
- 2. REINFORCING STEEL: GRADE 60 MIN. YIELD STRENGTH 60,000 P.S.I. SHALL BE EPOXY COATED AND AS PER SP 509.
- 3. LONGITUDINAL CONSTRUCTION JOINTS RECOMMENDED FOR STAGE CONSTRUCTION SHALL BE AS PER SP 5112 AND SHALL BE TREATED IN ACCORDANCE WITH SP 5108.
- 4. CROWN SHALL CONFORM TO THAT OF THE BRIDGE DECK. IF THE RATE OF CROWN OF THE BRIDGE DECK DIFFERS FROM THAT OF THE APPROACH ASPHALT PAVEMENT, A SMOOTH TRANSITION SHALL BE PROVIDED ON THE APPROACH ASPHALT PAVEMENT AT A TRANSITION RATE OF 200.
- 5. TRANSVERSE JOINT DETAILS AT THE APPROACH PAVEMENT END OF THE APPROACH SLAB SHALL BE AS DESCRIBED ON OTC STANDARD DRAWING AS-5.
- 6. BASE MATERIAL SHALL BE SP 304 AGGREGATE BASE.
- 7. GROOVE AND SEAL WITH 705.04 AS PER QOT STANDARD DRAWING BP-2.1.
- 8. TYPE A WATERPROOFING SHALL NOT EXTEND ABOVE THE BOTTOM OF THE GROOVE INTO WHICH THE JOINT SEAL IS TO BE PLACED. IT SHALL BE APPLIED TO THE ENTIRE AREA OF THE ALIGNMENT OR SUPERSTRUCTURE WHICH COINCIDES INTO CONTACT WITH THE APPROACH SLAB.
- 9. THE "INTEGRAL CONCRETE CURB" SHALL BE PROVIDED ON THE NEW APPROACH SLAB AS SHOWN IN SECTION D-D.
- 10. 1" PERFORMED EXPANSION JOINT FILLER SHALL BE PER 705.03.
- 11. THE TWO 4" DIAMETER PVC CONDUITS WITH MULTI-CELL INNERDUCT SHALL COMPLY WITH SP 625.
- 12. FOR SECTIONS A-A', B-B', C-C' AND D-D' AND ADDITIONAL INFORMATION SEE OTC STANDARD DRAWING AS-5.
- 13. THE FOLLOWING ITEMS SHALL BE INCLUDED IN THE UNIT PRICE BID PER SQUARE YARD FOR SP 526, CLASS C CONCRETE, APPROACH SLAB, USING TYPE I CEMENT (T#12):
 - 1. ALL JOINTS, INCLUDING JEWEL HOLES, DOWELS, AND GROUT.
 - 2. GROUT, AND INTERFACIAL SEAL.
 - 3. PREFORMED EXPANSION JOINT FILLER.
 - 4. MIDSPAN BARBERS
 - 5. REINFORCING STEEL

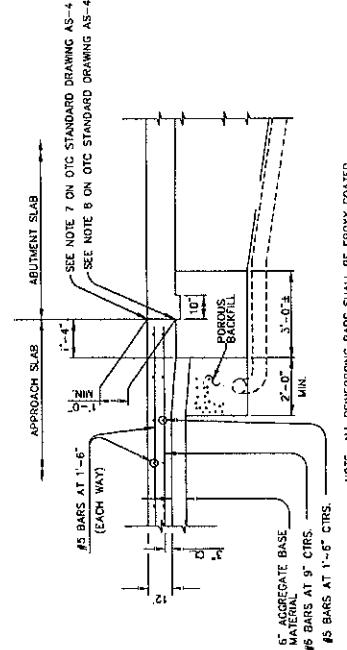


OHIO TURNPIKE COMMISSION

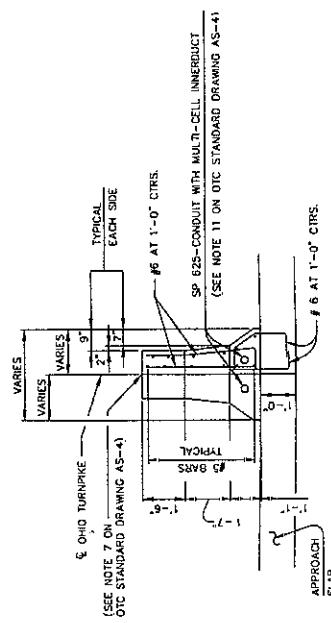
**REINFORCED CONCRETE
APPROACH SLAB —
FULL WIDTH REPLACEMENT**

**DATE NUMBER 20-2012-SM
O.T.C. STANDARD DRAWING AS-4**

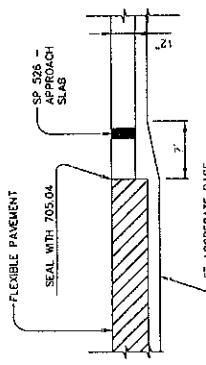
**PLAN
(SHOWING SKewed BRIDGE)**



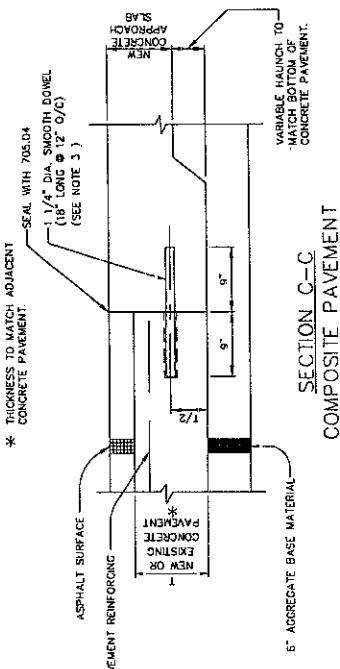
NOTE: ALL REINFORCING BARS SHALL BE EPOXY COATED.
SECTION A-A
(SEE NOTE 2)



SECTION B-B
NOTE: REINFORCING AND DIMENSIONS SYMMETRICAL ABOUT CENTERLINE

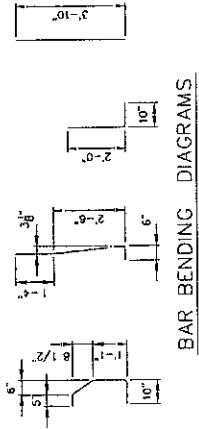


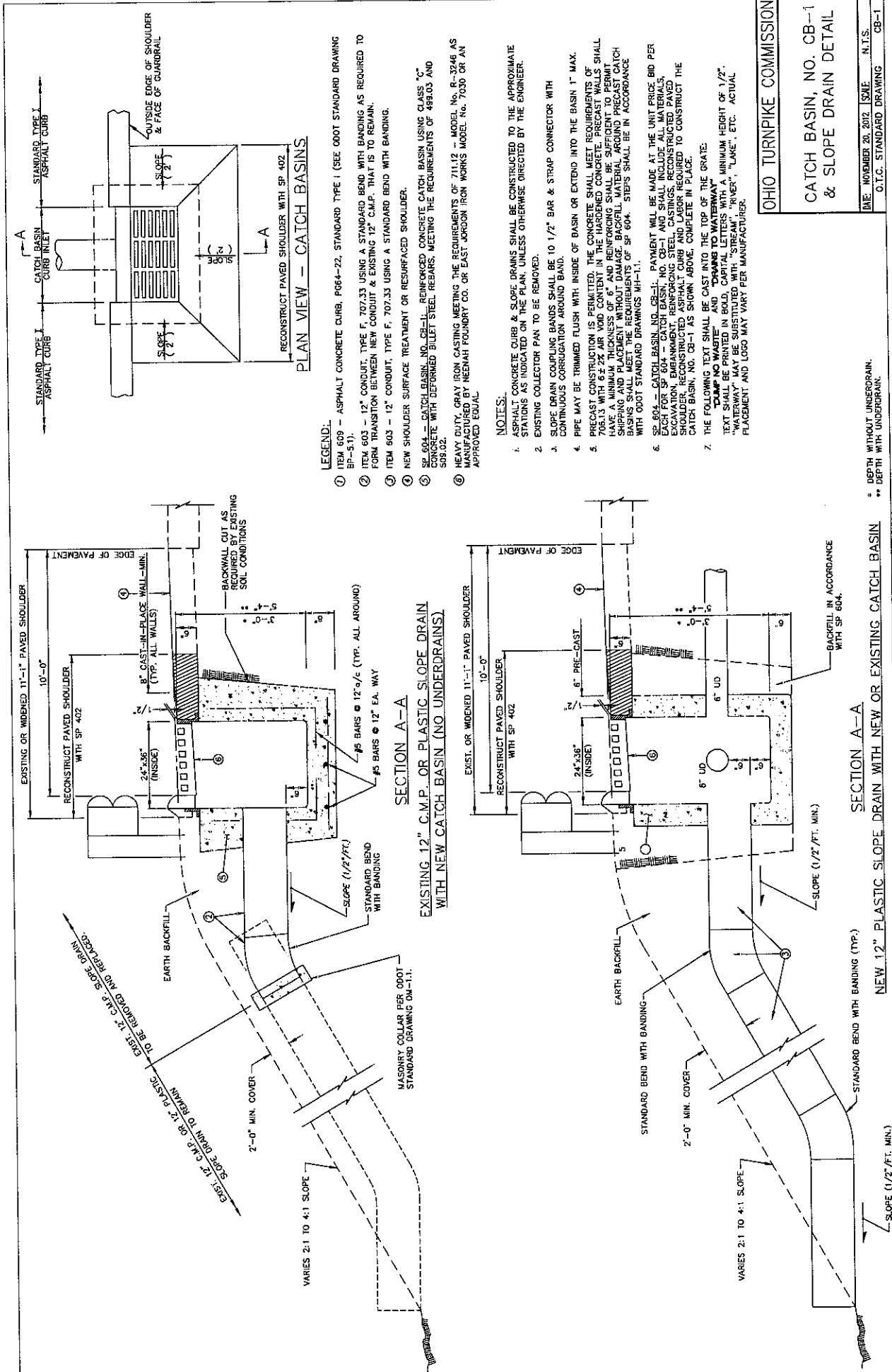
SECTION C-C
FLEXIBLE PAVEMENT



NOTES:
1. FOR LOCATIONS OF SECTIONS 'A-A', 'B-B', 'C-C', AND 'D-D' AND ADDITIONAL NOTES, SEE OTC STANDARD DRAWING AS-4 OR AS-4.
2. THE POROUS BACKFILL AND THE DRAIN PIPES ARE PROVIDED IN THE NEW WIDENED SECTION ONLY. SEE ADDITIONAL DETAILS FOR ADDITIONAL INFORMATION.
3. DRILL 1 3/4" DIA. HOLE INTO EXISTING CONCRETE, PARTIALLY FILL WITH NONSHRINKING GROUT BEFORE INSERTING PIVOT PER SP 5203 AND THE COST IS INCIDENTAL TO THE COST OF SP 5206 (APPROACH SLAB).

OHIO TURNPIKE COMMISSION
REINFORCED CONCRETE APPROACH SLAB SECTIONS AND DETAILS - FULL WIDTH REPLACEMENT
DATE: NOVEMBER 20, 2012 ISSUE: N.I.S.
O.T.C. STANDARD DRAWING AS-5





NOTES

GENERAL: Longitudinal joints shall be used when specified on the project section and shall be constructed as shown on this drawing in Zones 451 and 452 Pavement and Item 305 Base. The joint shall be on the centerline of the pavement unless otherwise shown. In zones where the movement width exceeds 1/2 in., the longitudinal joint shall be introduced into the concrete construction longitudinally at 1/2 in. intervals. The joint shall be a 5% REBOL deformed bars. A satisfactory device shall be used to hold the tie bars in proper position or they may be installed by a mechanical installing device. Tie bars shall not be centered on the longitudinal joint as nearly as practical.

BUTT JOINT: The longitudinal joint between adjoining slabs poured in separate operations shall be a butt joint with hook bolts or tie bars, unless otherwise shown on the plans. Bent tie bars shall not be permitted.

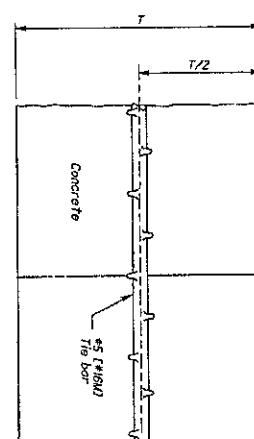
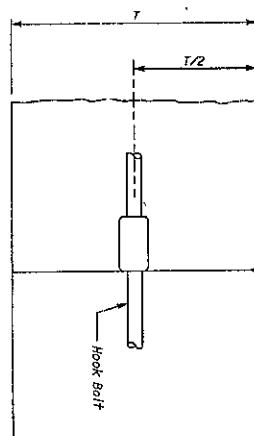
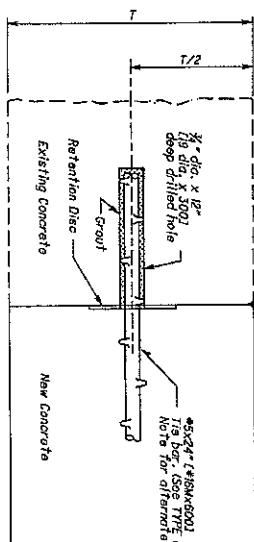
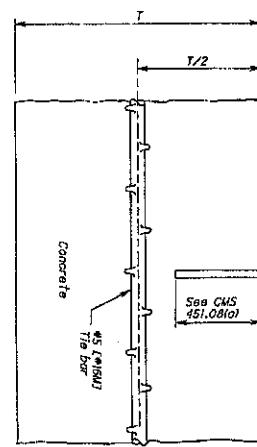
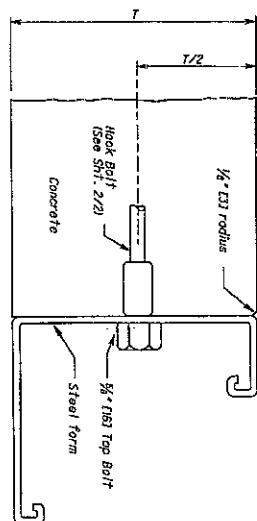
TYPE D DRILLED TIE LONGITUDINAL JOINT: Type D joints shall be constructed in accordance with CUS 255-05. Tie plates or plastic retention discs shall be clear or opaque white in color. Grout shall meet the requirements of CUS 255-02, Sec 1151 expansion anchors, FF-S-325, Group III, Type I or Group II deform bar and shall be installed according to the manufacturer's recommendations.

The use of self drilling expansion shield anchors, FF-S-325, Group III, Type I (or and tie bar) shall not be permitted.

See Sheet 2/2 for additional details.

SAWED JOINT

ACCEPTABLE METHOD OF FORMING JOINT

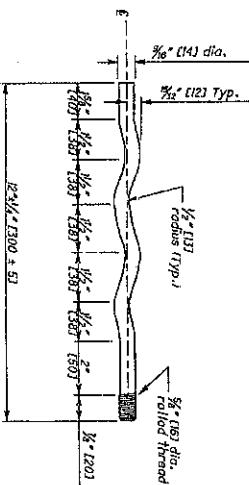
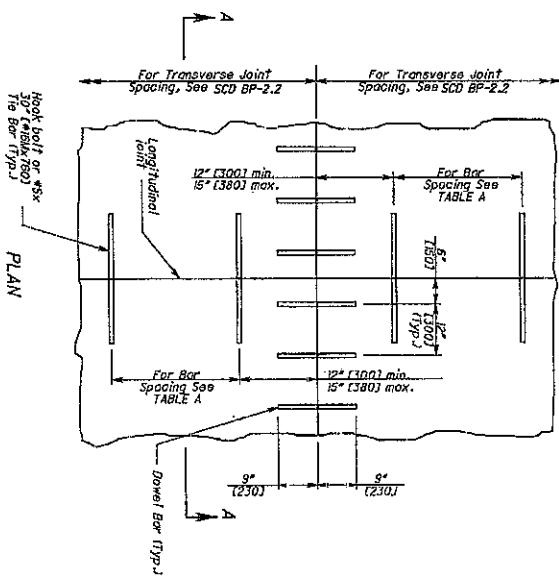
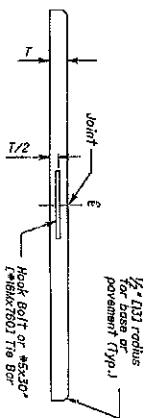


TYPE D (DRILLED TIED LONGITUDINAL) JOINT

BUTT JOINT
w/ HOOK BOLT

BUTT JOINT
w/ TIE BAR

TABLE A			
Thickness of Pavement	Transverse Joint Spacing	Number of Tie Bars per Sq. ft.	Max. Spacing Between Bars
10' (3050) or less	15' (4572)	7	28" (6607)
21' (6350)	10	28" (6607)	
35' (10660)	9	20" (5080)	
over 35' then 10' (3050)	21' (6350)	13	20" (5080)



Steel grouting to provide full load strength
1/8" (14) dia.

NYLON OR PLASTIC GROUT RETENTION DISCS FOR DOWEL/TIE BARS

1/8" (14) min. thick

NOTES

ENGAGE Edges joints with a thin metal edge having a radius of 1/8" (14). Any impression left in the surface of the pavement by the flat part of this engaging tool should be eliminated.

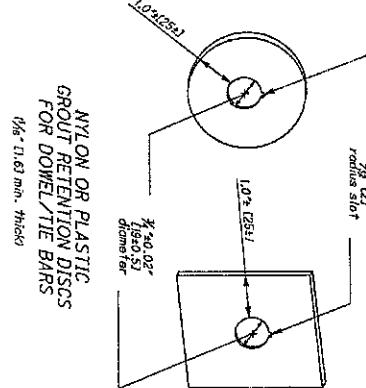
In lieu of the above method, the longitudinal joint may be constructed in accordance with CMA 151.010.

HOCK BOLTS: Threaded hook bolts and alternatives shall be turned to a tight fit when installed in couplings. Ensure the coupling is hooked on the same side of the joint as the shorter 1/8" (14) hook bolt.

METAL STRIPS: Tie bars, hook bolt assemblies, and the hook bolt alternate shall have a minimum strength of 11,000 pounds (49 kN).

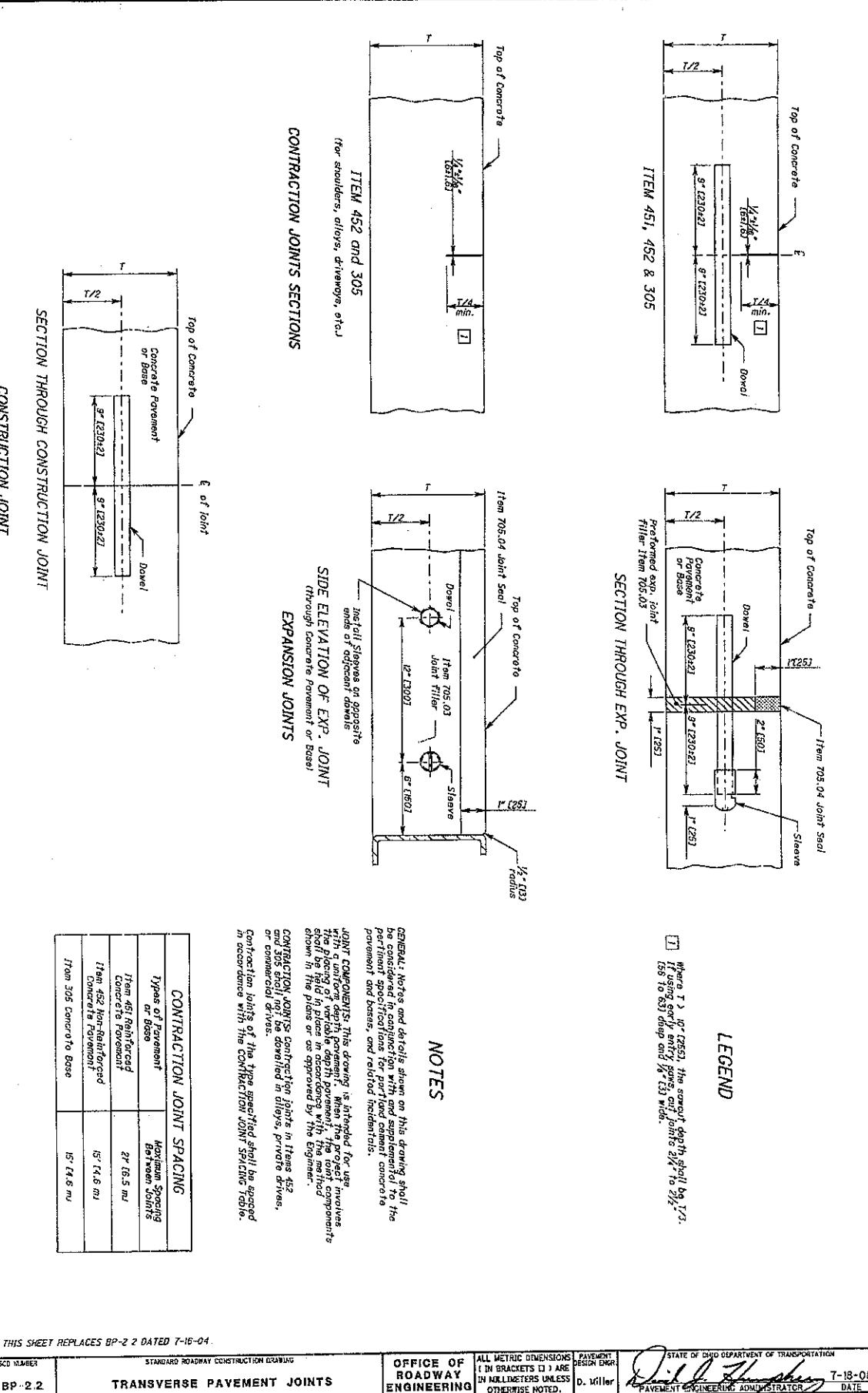
SPACING: Tie bars shall not be located within 12" (300) of any transverse joint.

SECTION A-A



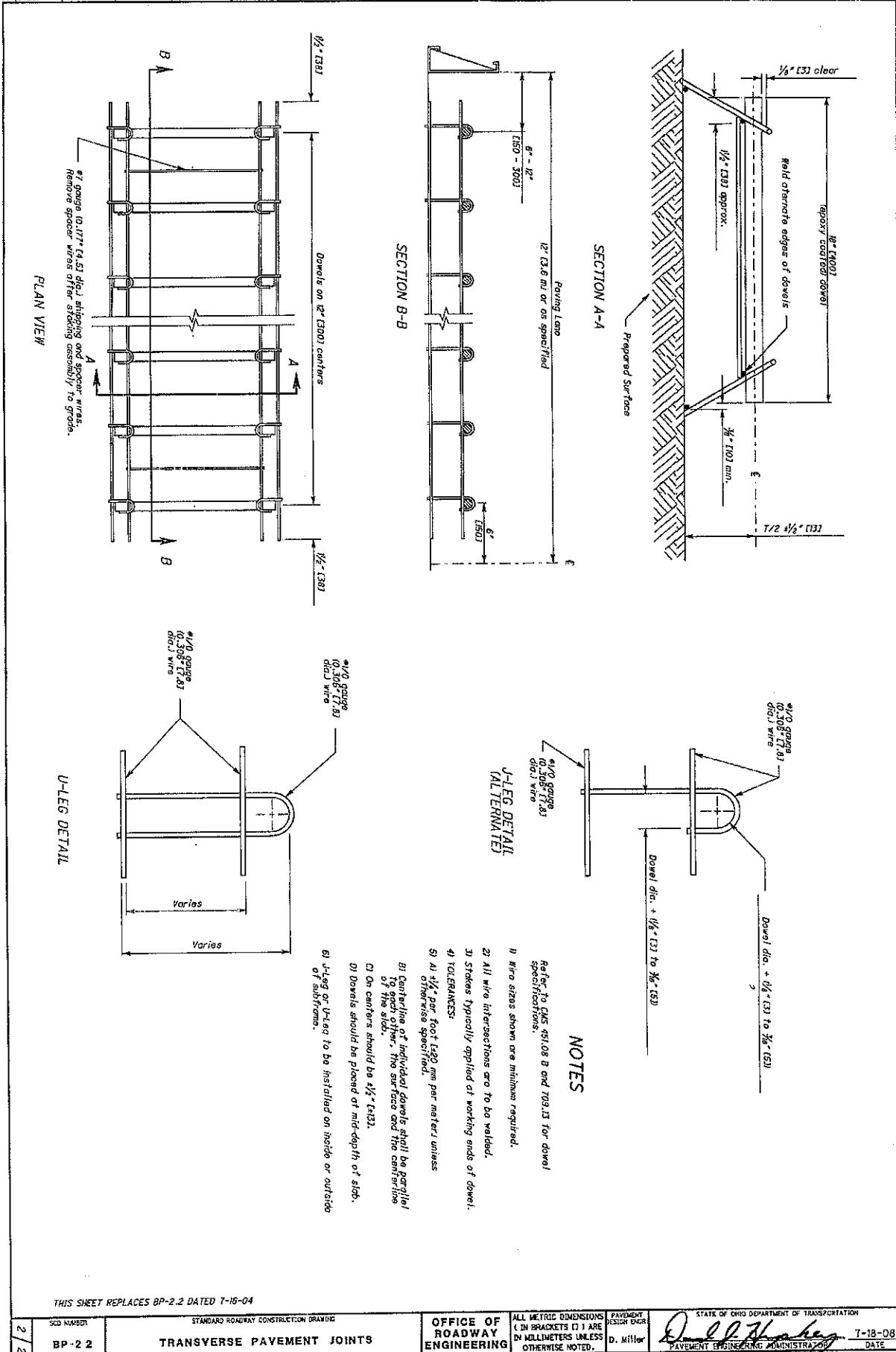
THIS DRAWING REPLACES BP-2.1 DATED 7-18-04.

SCD NUMBER BP-2.1	STANDARD ROADWAY CONSTRUCTION DRAWING LONGITUDINAL PAVEMENT JOINTS	OFFICE OF ROADWAY ENGINEERING	ALL METRIC DIMENSIONS (IN BRACKETS) ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.	PAYOUT DESIGN ENGR. D. MILLER	STATE OF OHIO DEPARTMENT OF TRANSPORTATION PAVEMENT ENGINEERING ADMINISTRATION DATE 7-18-08
2 / 2					



THIS SHEET REPLACES BP-2 DATED 7-16-04.

SCD NUMBER	STANDARD ROADWAY CONSTRUCTION DRAWING	OFFICE OF ROADWAY ENGINEERING	ALL METRIC DIMENSIONS (IN BRACKETS) ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.	PAVEMENT DESIGN ENGR:	STATE OF OHIO DEPARTMENT OF TRANSPORTATION PAVEMENT ENGINEERING ADMINISTRATOR	DATE
BP-2.2	TRANSVERSE PAVEMENT JOINTS	D. Miller	D. Miller	D. Miller	D. Miller	7-18-08



GENERAL. THIS DRAWING PROVIDES DESIGN AND CONSTRUCTION DETAILS. THE PROJECT PLANS SHALL SHOW THE LOCATION OF SPLICES PLUS A REFERENCE TO THIS DRAWING FOR PERTINENT DETAILS AND NOTES FOR SPLICING SPANS OF DIFFERENT SIZES OR WHERE SPLICES ARE LOCATED AT BEAM BEND POINTS. THE PROJECT PLANS SHALL INCLUDE SUFFICIENT DETAILS SUPPLEMENTING THIS DRAWING TO COMPLETELY DESCRIBE THE SPLICE.

DESIGN SPECIFICATIONS. THIS DRAWING CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1952, INCLUDING

DESIGN METHODS LOAD FACTOR DESIGN

ALLOWABLE STRESSES	STRUCTURAL STEEL	ASTM	A-709 GRADE 36	YIELD STRESS - 36 KSI
100000 psi	100000 psi	100000 psi	100000 psi	100000 psi

DESIGN, FOR EACH STRUCTURE THE DESIGNER SHALL CHOOSE A SPlice LOCATION AND DETERMINE THE MAXIMUM STRESSES (MOMENT AND SHEAR) AT THAT POINT. IN CONTINUOUS SPANS, SPLICES PREFERABLY SHALL BE MADE NEAR POINTS OF CONTRAFACTOR, THE SPLICE SHALL BE DESIGNED FOR NOT LESS THAN 1 1/2 THE AVERAGE DEFLECTION STRENGTH AT THE POINT OF SPLICE AND THE STRENGTH OF THE BEAM AT THE SAME POINT.

12. THE MODIFIED MAXIMUM STRESS SPECIFIED IN THE FATIGUE UNIT STRESSES NOTE, OR 1 1/2 THE STATIC STRENGTH OF THE BEAM, THE SPLICE DESIGNS SHOWN HEREIN ARE DESIGNED FOR 1 1/2. SEE NOTE FOR DESIGN LOADS. IF STRESSES 1 1/2 ARE MORE CRITICAL, THIS DESIGN SMALL NOT BE USED AND ANOTHER SPLICE SHOULD BE DESIGNED TO MEET THE ESTABLISHED REQUIREMENTS. THE STATIC BEAM STRENGTH AND THE SPLICE IS BASED ON THE NET SECTION FOR BENDING AND THE GROSS SECTION FOR SHEAR USING THE SAFETY UNIT STRESSES. WHEN SPLICING BEAMS OF DIFFERENT SIZES, THE SPLICE DESIGN SHALL BE BASED ON THE LIGHTER WEIGHT BEAM.

DESIGN LOADS, DESIGN MOMENT (KIP-FT) = 0.75 ($\frac{F_y}{d/2}$)

HIGH STRENGTH BOLTS ASTM A-325 DESIGN SLIP RESISTANCE IS BASED ON THE AASHTO CLASS A MINIMUM SLIP COEFFICIENT OF .35 DESIGN SLIP RESISTANCE = $2.15 F_y$ ($\frac{F_y}{d/2}$)^{0.5}

DESIGN SHEAR $(K_{IP1} = 0.75)$ $(0.58 F_y T_w (d - 2 \pi r))$

**REMOVED EXCEEDING 15% OF THE
GROSS-SECTION IS DEDUCTED (IN')
(SEE ASHSTO 10,18,11)**

d = MEMBER DEPTH LIM
 Tw = WEB THICKNESS LIM
 Tr = FLANGE THICKNESS LIM

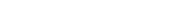
FATIGUE STRESSES. THIS SPICE STANDARD HAS NOT BEEN EVALUATED
TW = WEB THICKNESS [IN]
TF = FLANGE THICKNESS [IN]

FATIGUE STRESSES. THIS SPLICE STANDARD HAS NOT BEEN EVALUATED.
IS REQUIRED TO CALCULATE THE MAXIMUM MOMENT RANGE AND EVALUATE

IS REQUIRED TO CALCULATE THE MAXIMUM MOMENT RANGE AND EVALUATE ALLOWABLES GIVEN IN ASHTO TABLE 10.3.1A.

FASTENERS. FOR GRADE 30 OR STON STEEL, USE 1/4" DIAMETER HIGH STRENGTH BOLTS, 11.09 (ASTM A325) FOR GRADE 50 OR STON STEEL, USE 1/4" DIAMETER HIGH STRENGTH BOLTS, 11.09 (ASTM A325)

VERTICAL CLEARANCE. FOR GRADE SEPARATION STRUCTURES AN ALLOWANCE OF $\frac{3}{8}$ INCHES PLUS THE THICKNESS OF THE OUTSIDE FLANGE SPLICE PLATE SHALL BE USED IN COMPUTING THE ACTUAL VERTICAL CLEARANCE UNDER A BEAM SPLICE.

STANDARD BOLTED BEAM SPLICE FOR STEEL BEAM BRIDGES	EDITIONS QZ-19-92 <hr/> DESIGNED GEA <hr/> checked SAM <hr/> REFERENCE <hr/>	<hr/> <hr/> <hr/> <hr/> <hr/> BS-1-93	STATE OF OHIO DEPARTMENT OF TRANSPORTATION  12-19-94 <small>DATE</small>		DESIGN AGENCY OFFICE OF STRUCTURAL ENGINEERING
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STANDARD

BOLTED BEAM SPLICE

FOR STEEL BEAM BRIDGES
(FOR 50 KSI STEEL)

FOR 50 KSI STEEL

J / J